

SUSTAINABLE ENERGY AND MARINE PLANNING

UNDER THE AUSPICES OF THE ISSMGE (TC-308, TC-209) AND VSSMGE

OCTOBER 24

2022

HO CHI MINH CITY VIETNAM

ORGANISERS



SPONSORS









CONTENTS

VE	NUE3	3
INT	RODUCTION4	ŀ
PRO	DGRAM5	5
SPC	DNSORS	5
PRESENTATIONS		
A g	Accelerating transition to net zero through ocean-based solutions: lessons learned and good practice	3
P V	Planning, policy and integration for sustainable development of offshore wind energy in /ietnam 2022 - 2050)
lı	nnovative research to support offshore renewable energy10)
D	Data-driven ground models: the road to fully-integrated site characterization and design.11	L
٧	/ietnam OSW project development: key policy and framework issues)
F	rom nearshore to offshore – opportunities and challenges14	ŀ
lı	nnovation within offshore site investigation15	>
C	Offshore wind power: the potential renewable energy & remarkable aspects16	5
С р	Offshore wind energy challenges and solutions: a few lessons from nearshore wind power projects in Vietnam	,
h	nstallation risks and opportunities for future WTG foundations	3
Ρ	Potential development of floating offshore wind turbine in Vietnam offshore)
A	Al-based framework to predict wave-induced settlement of offshore wind turbines20)
R	Role of geotechnics in risk management of offshore windfarm projects	L
lı v	nvestigation of the long-term cyclic behaviour of monopile foundation by impact and /ibratory installation	2
C C	Dptimisation of capacity predictions for driven Piles performance in carbonate silts for offshore structures in the Arabian Gulf23	3
E	experimental investigation of wave scattering around a large vertical circular cylinder24	ŀ
C	Case studies on mitigating pile foundation refusals25	;
Т	ra Vinh No.3 windfarm: CBOP package - lessons learned	5
F	lighlights from R&D on innovative monopile installation and decommissioning27	7





VSOE2021 – The Second Vietnam Symposium on Advances in Offshore Engineering Sustainable Energy and Marine Planning **24 October 2022, Ho Chi Minh City, Vietnam**

https://vsoe2021.sciencesconf.org/



VENUE

Morning Session (Oct 24 th)	Lunch and Afternoon Session (Oct 24 th)
https://oisp.hcmut.edu.vn/en/	https://saigon.newworldhotels.com/en/
268 Ly Thuong Kiet Street Ward 14	
District 10. Ho Chi Minh City. Vietnam.	76 Le Lai Street, District 1, Ho Chi Minn City,
	Vietnam



INTRODUCTION

Following the success of the first Vietnam Symposium on Advances in Offshore Engineering (VSOE2018), the second Symposium, <u>VSOE2021</u>, which was delayed due to the COVID19, will be held in Ho Chi Minh City, Vietnam in 24 October 2022. The second VSOE event is being organised by the Association of Vietnamese Scientists and Experts (<u>AVSE Global</u>) in collaboration with Ho Chi Minh City University of Technology (<u>HCMUT</u>) and the Vietnam Administration of Seas and Islands (<u>VASI</u>). The event is organised in parallel with the Joint International Conference on Environment, Earth Science and Sustainability (<u>ICES</u>).

Focusing on the theme of "Sustainable Energy and Marine Planning", VSOE2021 aims to enhance the sustainable use of our marine resources while ensuring the health of the ecosystem as well as the effective management of marine activities including energy production and infrastructures.

VSOE2021 continues providing a platform for all participants to exchange knowledge and experience gained recently in offshore engineering, technology innovations, and marine spatial plan to achieve the goal of economic, reliable and sustainable solutions for offshore energy development, and ecosystembased management of the marine environment. VSOE2021 intends to bring together researchers, practitioners, policymakers, and entrepreneurs to discuss and promote technology and policy changes toward renewable energy, as well as to generate business opportunities in offshore energy, both domestically in Vietnam and globally.

We have received a tremendous amount of support from a diverse group of participants all over the world. More than 170 abstracts were submitted in the first phase and 100 full papers were submitted in the second phase. Despite our tough review process in which each paper was reviewed by at least two relevant experts, 60 papers have been accepted and published online by the international publisher Springer as a volume in the Lecture Notes in Civil Engineering series, indexed by SCOPUS.

We would like to acknowledge the wonderful support of the scientific committee and the invited experts, who have all spent their valuable time and made tremendous efforts to review the papers. We are grateful to the valuable support from our sponsors: FECON Corporation (Vietnam), NUCE (Vietnam), Sarathy Geotech (SGES) (India) and CTE WIND (Vietnam).

You are cordially invited to attend VSOE at <u>HCMUT premises</u> (morning session), and at <u>New World</u> <u>Saigon Hotel</u> (afternoon session) on October 24th, 2022.

We believe that the symposium will provide attendees with the recently collected and valuable knowledge from experts on topics that include offshore engineering, technology innovations, and offshore wind.

Please visit our website for registration and further details: https://vsoe2021.sciencesconf.org/

We wish you good heath, success and prosperity.

Dr Hong DOAN (EDF, France) & Prof. Van Thang LE (HCMUT, Vietnam) & Dr Khoa D.V. HUYNH (NGI, Norway)

On behalf of the VSOE Organising and Scientific Committees Website: https://vsoe2021.sciencesconf.org/ E-mail: vsoe@avseglobal.org



PROGRAM

8h00 onwards	REGISTRATION (24-Oct-2022 @HCM Uni. of Technology)
8h30	Welcome & Opening Address: VSOE/AVSE + HCM Uni. of Technology + VASI + Trent Uni. + IIES Director
8h30	Keynote: Representatives from MONRE and from MOIT (TBC)
8h50	Keynote : Ambassador Hilde Solbakken, The Norwegian Ambassador to Vietnam Accelerating transition to net zero through ocean-based solutions: lessons learned and good practice
9h05	Keynote : Prof. Ong Choon Nam, Saw Swee Hock School of Public Health, National University of Singapore Multidisciplinary Research for a Sustainable Environment
9h50	BREAK / EXHIBITION / POSTER SESSION (@HCM Uni. of Technology)
10h00	Keynote: Prof. Margaret Graham, The University of Edinburgh
10h45	Keynote: Ha Duong Minh, Vietnam Initiative for Energy Transition (VIET) Planning, policy and integration for sustainable development of offshore wind energy in Vietnam 2022 - 2050
11h15	Photo sessions, Exhibitions, Poster and Coffee Break (@HCM Uni. of Technology)
11h30	TRANSPORT TO NEW WORLLD SAIGON HOTEL & LUNCH
13h00	PLENARY SESSION & PANEL DISCUSSION (New World Saigon Hotel - Club Boardroom)
13h00	Keynote : Prof. Phil Watson, The University of Western Australia Innovative research to support offshore renewable energy
13h20	Keynote: Maarten Vanneste, Norwegian Geotechnical Institute Data-driven ground models: the road to fully-integrated site characterization and design
13h45	Panel Discussion: Vietnam Offshore Energy Moderators: Hang Dao, Clean Energy Investment Accelerator Vietnam & Ha-Duong Minh, VIET Denzel Eades, Managing Director, Pioneer International Consulting, Singapore Dung Vu Viet, Director, Power Plant Operation and Management Center, PECC2 Riccardo Felici, Country Manager, OWC Vietnam David Donaghy, Technical Manager - Geotechnics, Ocean Infinity Dien Tran Quoc, Deputy General Director, PECC3 Hung Nguyen Viet, CEO, CTV Wind
15h15	BREAK



15h30	TECHNICAL PRESENTATIONS (New World Saigon Hotel - Club Boardroom)
15h30	Daniele Bertalot, Geowynd, UK Installation risks and opportunities for future WTG foundations
15h40	Pham Thanh Dam, Duy Tan University Potential Development of Floating Offshore Wind Turbine in Vietnam offshore
15h50	Amir Moghaddam, School of Engineering, RMIT University An Al-based framework for predicting liquefaction-induced deformation of Offshore Wind Turbines
16h00	Indrasenan Thusyanthan, Managing Director Gavin & Doherty Geosolutions, Ireland Role of geotechnics in risk management of Offshore Windfarm Projects
16h10	Le Viet Hung, Technische Universität Berlin, Germany Investigation of the long-term cyclic behaviour of monopile foundation by impact and vibratory installation
16h20	Muhammad Bilal Mumtaz, Fugro Survey Middle East (UAE) Optimisation of Predictions for Driven Piles Performance in Carbonate Silts for Offshore Structures
16h30	Mai Cao Tri, Hanoi University of Civil Engineering Experimental Investigation of Wave Scattering Around a Large Vertical Circular Cylinder
16h40	Sumanth Haribhat Chandrashekhar, Sarathy Geotech and Engineering services pvt Ltd., India Case studies on Mitigating Pile Foundation Refusals
16h50	Pham Duc Huyen, FECON, Vietnam Tra Vinh no.3 Windfarm: CBOP Package - Lessons Learned
17h00	Ahmed Elkadi, Research Program Manager: Energy Transition, Deltares, Netherlands Highlights from R&D on innovative monopile installation and decommissioning
17h15	Closing Ceremony: VSOE/AVSE + HCM Uni. of Technology + VASI Representatives
18h45	GALA DINNER (Floating Restaurant Indochina Junk - Cruise along Saigon River)

SPONSORS











PRESENTATIONS

Lecture Notes in Civil Engineering

Dat Vu Khoa Huynh Anh Minh Tang Dinh Hong Doan Phil Watson *Editors*

Proceedings of the 2nd Vietnam Symposium on Advances in Offshore Engineering

Sustainable Energy and Marine Planning

🖄 Springer

https://link.springer.com/book/10.1007/978-981-16-7735-9



Accelerating transition to net zero through oceanbased solutions: lessons learned and good practice

Hilde Solbakken

Norwegian Ambassador to Vietnam The Royal Norwegian Embassy in Hanoi

Norway's economic reliance on its oceans is pivotal, with ocean-based industries contributing significantly to its welfare and export earnings. As a leading figure in ocean research, responsible marine resource management, and a major player in shipping and seafood export, Norway is actively transitioning from traditional oil and gas production to renewable energy. This keynote speech, presented against the backdrop of COP26 and Vietnam's ambitious net-zero goals, shares Norway's experience and best practices in leveraging ocean-based solutions to combat climate change. It explores how such strategies could contribute one-fifth of the necessary annual greenhouse gas emissions reductions by 2050, keeping global temperature rise below 1.5 degrees Celsius.

The speech delves into five key ocean-based climate action areas: investing in nature-based solutions, harnessing ocean-based renewable energy, decarbonizing ocean industries, securing sustainable future food sources, and implementing carbon capture and storage in the seabed. The Norwegian experience in developing offshore wind power is highlighted, showcasing the country's ambitious targets and the synergies between the maritime and energy sectors in this green transition. The role of integrated ocean planning is emphasized as a crucial framework for balancing environmental, industrial, and societal needs.

Overall, the speech underscores the criticality of a healthy ocean in the global fight against climate change, presenting a comprehensive approach that combines policy, industry innovation, and sustainable practices to achieve a prosperous, environmentally-resilient ocean-based economy.



Planning, policy and integration for sustainable development of offshore wind energy in Vietnam 2022 - 2050

Ha Duong Minh

Vietnam Initiative for Energy Transition (VIET), Vietnam





Planning, policy and integration for sustainable development of offshore wind energy in Vietnam 2022-2050

Dr. Minh Ha-Duong

2022 Vietnam Symposium on Avances in Offshore Engineering Ho Chi Minh City, 24 October 2022

Regulations Planning Biomass Wind Grid

Credibility Excellence Happiness Responsibility Interdependence VIET

an active INDEPENDENT THINK TANK since 8/2018 Research Expertise Consultancy Training

Dialogue Scenarios Modeling Economics Integrated assessment International experience



1. Wind power in Vietnam today

88 projects operating4.2 GW installed capacityNo "real" offshore under construction



Vietnam installed 40GW of power generation capacity in 2010-2021



3









5

6

Source: Author. Dataset available at Zenodo.org



Mean wind power density for Vietnam at 100m, color scale from 0 to 1000+ W/m2

Source: Global wind atlas 3.1





Wind farms in Vietnam Source: pgis.vietse.vn

2. Vietnam needs more electricity

And wind is the #1 clean, affordable solution







Source: Our World in Data based on BP Statistical Review of World Energy (2022); Our World in Data based on Ember's Global Electricity Review (2022); Our World in Data based on Ember's European Electricity Review (2022) OurWorldInData.org/energy • CC BY



Information about failures of some substations in the North on 4 July 2022

According to information from the National Load Dispatch Center, at 13:00 on 4 July 2022, voltage fluctuations appeared on the Northern power system, affecting some customers' power supply in the North.

The initial cause was determined hot weather, which increased power consumption and some generator units failed causing voltage fluctuations, then power supply interruption resulted in some customers in the North.

Immediately after the failure occurred, the National Load Dispatch Center actively coordinated with power generation and grid operating units to troubleshoot the problem and restore power supply to customers as quickly as possible.

By 15:00 on the same day, all customers who were affected by power disruption had been restored to the power supply and the Northern power system have returned to stable operation.





11

12

Undeflated prices. *bp* Statistical Review of World Energy June 2022 charting tool.



Undeflated prices. *bp* Statistical Review of World Energy June 2022 charting tool.



Energy sector CO2 emissions must peak before 2035

No new coal after 2030 Control methane emissions Carbon neutrality by 2050

15

Wind in Vietnam's power development plan

draft 6328/TTr-BCT 13/10/2022





3. Challenges ahead

Integrate, manage variability Finance, find capital Connect, build the transport infrastructure



Wind generation variability challenge



Regions of Vietnam, first week of January, generation profile from Renewable Ninja.

16



Power mix simulation for the Red river delta (2030) in a very windy week and high renewables scenario



Windless week \rightarrow Thermal power here in 2030





Power sector capital challenge

9.0 - 12.6 billion USD for generation sources + 1.5 - 1.6 billion USD for the grid

per year, over 2021-2030

According to 5709/TTr-BCT 23/09/2022

Infrastructure challenge







Fig. 1. Load flow snapshot in 2023, 2025 and 2030 (from left to right) - Cases of high renewable sources including max capacity from <u>OWFs.</u>

Conclusion Vietnam V Offshore wind

- Future of domestic power supply
- Hydrogen production potential
- Growth sector with a global supply chain



More info at https://vietse.vn

























Thank you!





7 Floor, 18 Ly Thuong Kiet Str. | Hanoi | 100000 T : +84.243.204.5554 | M: +84.945.336.677 | info@vietse.vn | VIETSE.VN | Facebook | Linkeln

https://vsoe2021.sciencesconf.org/



Innovative research to support offshore renewable energy

Phil Watson The Centre for Offshore Foundations Systems (COFS) The University of Western Australia

Innovative research to support offshore renewable energy



Phil Watson – The Centre for Offshore Foundations Systems (COFS) with input from ... Fraser Bransby, Christophe Gaudin, Britta Bienen, Conleth O'Loughlin, Mike O'Neill, Alessio Mentani plus PhD students as acknowledged on individual slides







Innovative research to support offshore renewable energy



Phil Watson – The Centre for Offshore Foundations Systems (COFS) with input from ... Fraser Bransby, Christophe Gaudin, Britta Bienen, Conleth O'Loughlin, Mike O'Neill, Alessio Mentani plus PhD students as acknowledged on individual slides



New challenges ...

- Foundations
 - Dynamically sensitive (stiffness)
 - Low V/M ratio
 - New regions (soils)
- Field layout
 - Spatial variability
 - Shared anchors (floating)
- Installation
 - Noise
 - Low t/D ratios
 - Highly loaded spudcans
- Seabed
 - Scour
 - Cables (thermal)

... and much, much more!









Foundations

3





THE UNIVERSITY OF

AUSTRALIA

Suction buckets



Innovative installation strategies

Challenges:

- Cavitation limit
- Buckling limit

Strategies:

- Pressure cycling
- Tip water injection





Suction buckets





Other examples of ongoing modelling





New design tools







Field layout

9

Geotech parameters from geophysics?











Installation

https://vsoe2021.sciencesconf.org/



THE UNIVERSITY OF

WESTERN AUSTRALIA

Vibro-installation & loading (monopiles)

Fast installation, less fatigue, reduced noise, economic

Creation of an innovative mini vibro-driver for centrifuge

Challenge: scaling dynamic events!

Field f \approx 23 Hz \rightarrow N = 100 \rightarrow Centrifuge f = 2300 Hz <u>Main goals:</u> study mechanisms of vibro-driving and impact on lateral capacity

Vibro-installation

Cycle-by-cycle analysis of penetration











THE UNIVERSITY OF

WESTERN AUSTRALIA

Pile damage during driving

Driving though scour protection or with boulders

Centrifuge and numerical studies Low t/D can lead to failures







Pile driving in centrifuge



Juliano Nietiedt (now Fugro)







Scour

15

Spudcan penetration due to scour











In closing



Offshore wind

- is well established in the North Sea, and rapidly expanding
- presents challenges that differ from conventional oil & gas projects
- has low margins to work with
- is putting pressure on supply chains

Geotechnical solutions need to be efficient, robust and economic – while considering fabrication and environment constraints

It is an exciting time for both researchers and practitioners!

17



Data-driven ground models: the road to fullyintegrated site characterization and design

Maarten Vanneste

Norwegian Geotechnical Institute, Norway

NG

Data-driven Ground Models: The Road to Fully-Integrated Site Characterization and Design

Maarten Vanneste, Guillaume Sauvin, Jean-Remi Dujardin, Carl Fredrik Forsberg, Rasmus T. Klinkvort, Cathinka S. Forsberg, Ragnhild C. Hansen [NGI, Oslo] Mark E. Vardy [SAND Geophysics, Southampton]

VSOE 2022-10-24

NG

Data-driven Ground Models: The Road to Fully-Integrated Site Characterization and Design

Maarten Vanneste, Guillaume Sauvin, Jean-Remi Dujardin, Carl Fredrik Forsberg, Rasmus T. Klinkvort, Cathinka S. Forsberg, Ragnhild C. Hansen [NGI, Oslo] Mark E. Vardy [SAND Geophysics, Southampton]

VSOE 2022-10-24

Outline

- Offshore Wind Market Outlook
- Current Developments Requirements on Geophysical and Geotechnical Site Investigations, and the need for Integration
- Data-driven Ground Models: site-wide predictions of geotechnical parameters and their uncertainties (examples)
- Looking ahead: Complementary data and methods



Geology

technics

Geo

physic







Outline

Offshore Wind – Market Outlook

- Current Developments Requirements on Geophysical and Geotechnical Site Investigations, and the need for Integration
- Data-driven Ground Models: site-wide predictions of geotechnical parameters and their uncertainties (examples)
- Looking ahead: Complementary data and methods





Market Outlook - Offshore Wind

- Green transition: transfer "reservoir" technology/methodology (and jobs) to near-surface characterization, adapted to different challenges
- Significant growth in offshore renewables (particularly OW)

shallow waters areas with fixed constructions Floating systems, suitable for deep-water environments

Key points

Fast-track solutions and early-phase decision gates

Extensive areas \rightarrow efficient

NGI characterization



* CAGR = Compound Annual Growth Rate Source: GWEC Market Intelligence, June 2020



Outline

- Offshore Wind Market Outlook
- Current Developments Requirements on Geophysical and Geotechnical Site Investigations, and the need for Integration
- Data-driven Ground Models: site-wide predictions of geotechnical parameters and their uncertainties (examples)
- Looking ahead: Complementary data and methods



NG



Offshore Wind – Current Developments





Offshore Wind Foundation Concepts



Geophysical SI: Characterization, Hazard & Risk

Prime hazards:

Weak layers; Shallow gas/fluid flow; Faulting/seismicity; Liquefaction; Bedforms/seabed mobility/scouring; Boulders; Loads; presence of Glauconite; UXO; ++

Spatial variability/stratigraphy implies uncertainty in soil properties (like strength; stiffness, etc.)

Establish 3D predictive model for engineering applications










Integration is an absolute requirement



Outline

- Offshore Wind Market Outlook
- Current Developments Requirements on Geophysical and Geotechnical Site Investigations, and the need for Integration
- Data-driven Ground Models: site-wide predictions of geotechnical parameters and their uncertainties (examples)
- Looking ahead: Complementary data and methods





NG





G³ Trilogy: Fundamental building blocks

Powerful complementary techniques, with pros and cons, and specific language

 Non-invasive Invasive Processes by which soils form and change over • 2D to 3D (# millions) ID (# tens) time Samples + CPT Seismo-acoustics Qualitative Resolution, z (cm) Ghost, multiples, time- Time and spatial scales depth conversions Empirical relations Geofantasies? Resolution, z(~m), Sample quality? decreasing with depth Essential for consistency *S*₁, γ, φ, G₀, e, ρ, σ and data extrapolation R, Z, V, Q, ρ, σ (large-strain) (small-strain) Geo-Geo-Geology technics physics N



Solution – Data-driven Ground Model



QGM #1: Unit-based parameter model



NG

NG



QGM #2: co-Kriging model



QGM #3: Inversion + ML





Aim: 3D prediction of CPT properties



Ground Model – Input in foundation design





Outline

- Offshore Wind Market Outlook
- Current Developments Requirements on Geophysical and Geotechnical Site Investigations, and the need for Integration
- Data-driven Ground Models: site-wide predictions of geotechnical parameters and their uncertainties (examples)
- Looking ahead: Complementary data and methods



NG



3D imaging can be accomplished!





Physics-based ML



- Acoustic Impedance inversion combined with ML (Temporal Convolutional Network)
- Successfully applied to complex models, and can be scaled to higher frequencies
- **NGI** Currently being be applied to OW settings



Dynamics – S-waves

- Correlation of Sonic Scanner data (giving slowness) and Cone Penetration Testing data (giving undrained strength)
- Application from shallow gas field in the North Sea
- Good relationship between S-wave slowness and undrained shear strength!
- Collect more data (e.g., MASW, SCPT)



Dynamics – S-waves









Summary

- Offshore Wind is key for the energy transition
- Development areas are complex, and multi-disciplinary data integration and innovation is essential: QGM with focus on uncertainty assessment can be used for engineering design (TNW qualified by DNV)
- Complementary geophysical (e.g., 3D, long-offsets, AVO, Shear waves) and geotechnical data (novel SCPT) should be acquired
- Open data bases (e.g., RVO model) allow significant R&D!





NG





NG



Vietnam OSW project development: key policy and framework issues

Denzel Eades

Pioneer International Consulting, Singapore

Pioneer International Consulting

Vietnam OSW Project Development Key Policy and Framework Issues



Pioneer International Consulting

24 October 2022



Pioneer International Consulting Vietnam OSW Project Development Key Policy and Framework Issues



Pioneer International Consulting

24 October 2022



Pioneer International Consulting



Contact

denzel.eades@pioneer-pic.com Vietnam: +84 (0) 77841 5257 Hong Kong: +852 9842 2281

Denzel Eades

CO-FOUNDER AND MANAGING DIRECTOR

Denzel is the Co-founder and Managing Director of PIC and manages the firm's offices in Vietnam and Singapore.

PIC is a strategic consultancy firm focused on energy transition and renewable energy project development in Vietnam. The firm brings over 25 years of experience in Vietnam's power sector and has been actively advising on Vietnam's onshore and offshore wind market since 2019.

Denzel brings over 17 years experience in project finance and development having worked with both global financial institutions and leading law firms in both Europe and Asia. Prior to founding PIC in 2018 he worked with Clifford Chance LLP, HSBC and MUFG Bank delivering financing solutions and transactions for leading corporates, private equity and project sponsors.

After qualifying with Clifford Chance in LLP in 2004 he had a global career with the firm based in London, Tokyo, Moscow, Hanoi and Bangkok. Following Clifford Chance he worked for HSBC in Hong Kong with a focus on distressed debt and global restructurings. He was based in Hanoi, Vietnam from 2010-2012 and was seconded to MUFG Bank's project finance team in Tokyo in 2013.

Having worked on some of the largest and country first project and leveraged financing in the UK and Asia he has broad and deep expertise in project, leveraged finance and complex global restructurings.

Denzel is a graduate of the London School of Economics (BSc Hons) and Kellogg School of Management MBA.





MONRE Report 126/BC-BTNMT dated 4 October 2022

https://vsoe2021.sciencesconf.org/





MONRE Report 126/BC-BTNMT dated 4 October 2022



5

Pioneer



7



PIC is a strategic consultancy firm focused on energy transition and renewable energy project development in Vietnam.

The firm brings over 25 years of experience in Vietnam's power sector and has been actively advising on Vietnam's onshore and offshore wind market since 2019.

Combining international and local expertise PIC provides strategic consulting, project development, transaction and financial advisory services across the project value chain.

Contact

Denzel Eades Managing Director Email: denzel.eades@pioneer-pic.com

Vietnam | 56 Pho Duc Chinh, Ba Dinh, Hanoi Hong Kong | Unit 1104, Crawford House, 70 Queens Road Central

Vietnam +84 24 320 43048 | Hong Kong +852 2892 7301

www.pioneer-pic.com





Experience in providing O&M service for wind power plant

Vu Viet Dung

Power Engineering Consulting Joint Stock Company 2 (PECC2), Vietnam







TABLE OF CONTENTS

ABOUT OUR COMPANY 01

> 02 **HEALTH, SAFETY AND ENVIRONMENT**



O&M SERVICES

2

PECC-2POM

https://vsoe2021.sciencesconf.org/



01 About our company



PECC-2POM

ABOUT PECC2POM

Over 37 years operating in the field of surveying, designing, monitoring and managing construction of power projects, PECC2 has continuously developed and become one of the leading brands in the energy industry in Vietnam. Foreseeing the development potential of the renewable energy industry in Vietnam, PECC2 has quickly launched the Power Plant Operation and Management service (PECC2POM) to meet the market's needs as well as contribute to boosting the development of the energy industry in Vietnam.



HIGHLIGHTS



Operating Power Plant Projects (Wind, Solar, Thermal)



Accident occurred or HSE violate happened in our power plant



The first licensed OCC in Vietnam



Are being generated every month



Availability Factor of our power plants

PECC-2POM



OUR PROJECTS

No.	PROJECTS	GRID VOLTAGE	CAPACITY	No.	PROJECTS	GRID VOLTAGE	CAPACITY
1	Loc Ninh 1 solar power project	220kV	200 MWp	16	Gio Thanh 2 solar power project	110kV	50 MWp
2	Loc Ninh 2 solar power project	220kV	200 MWp	17	Vinh Tan solar power project phase 1	22kV	6.2 MWp
3	Loc Ninh 3 solar power project	220kV	150 MWp	18	Mui Ne solar power project	110kV	40 MWp
4	Hoa Hoi solar power project	220kV	257 MWp	19	LIG Quang Tri solar power project	110kV	50 MWp
5	Son My 3.1 solar power project	110kV	50 MWp	20	Tan Thuan offshore wind power project	110kV	75 MW
6	Dau Tieng 1 solar power project	220kV	180 MWp	21	Kosy wind power project	220kV	40 MW
7	Dau Tieng 3 solar power project	220kV	180 MWp	22	Soc Trang 7 wind power project	110kV	30 MW
8	Ninh Phuoc 1&2 solar power project	110kV	58 MWp	23	Nhon Hoi wind power project	110kV	60 MW
9	Song Luy 1 solar power project	110kV	47 MWp	24	Yang Trung wind power project	220kV	145 MW
10	Phuoc Ninh solar power project	110kV	45 MWp	25	Cho Long wind power project	220kV	155 MW
11	Phong Dien 2 solar power project	110Kv	50 MWp	26	la Pet Dak Doa 1 &2 wind power project	220kV & 500kV	198 MW
12	Thien Tan 1.2 solar power project	220kV	100 MWp	27	Cam Hoa solar power project	110kV	50MWp
13	Thien Tan 1.3 solar power project	110kV	50 MWp	28	Chu Ngoc solar power project	22kV	15MWp
14	Nhon Hai solar power project	110kV	35 MWp	29	38 solar rooftops projects	22kV	30 MWp
15	Gio Thanh 1 solar power project	110kV	50 MWp	30	Duyen Hai 2 Coal Fired Thermal Power Plant (O&M of BOP)	500kV	1320 MW

PECC+2POM

THE FIRST OPERATION CONTROL CENTER (OCC) LICENSED IN VIETNAM, OPERATED AND MANAGED BY PECC2POM





https://vsoe2021.sciencesconf.org/



OUR WIND POWER PROJECTS



PECC-2



PECC-2POM



ISO SYSTEM





PECC-2POM



ZERO ACCIDENT AND ZERO FIRE OBJECTIVES: 1) To ensure zero human accident 2) To ensure zero equipment damage All of our personnel have to take Safety and Health training and have all required certificates: **Operational personnel:** + Electrical safety certificates 1) Occupational safety and health certificate 2) 3) Fire prevention and fighting certificates + Wind Turbine Generator Maintenance Expert 1) Above certificates as operational personnel Elevator operation certificate 2) 3) Certificate of Global Wind Organization (GWO)* + Crew transfer vessel operator: 1) Above certificates as operational personnel Safety Certificates of Authorized Organization by Ministry of Transport Viet Nam 2) (Such as: Sea Survival Certificates, etc.) Build emergency response procedures and incident rehearsal: 1) Fire prevention procedures Typhoon prevention procedures and rescue at sea 2)

PECC-2POM

HSE PROCEDURES AND PLANS APPLIED AT OUR WIND PROJECTS

1	Safety Procedure in Operation and Incident handling
2	Typhoon and flood prevention Procedure
3	Rescue plan for operators who fell into the sea
4	Rescue and transfer plan for injured personnel on WTG's foundation to the CTV
5	Rescue plan for operators stuck in the elevator
6	Rescue plan for operators on WTG's foundation when the CTV can't access the foundation in bad weather

PECC+2POM







Training necessary skills to ensure safety:

- + Method of traveling between CTV and wind turbine
- + Method of climbing stair when there is no elevator



PECC-2POM

15

CREW TRANSFER VESSEL AT OUR WIND POWER PLANT

CONTENT	SPECIFICATION
The CTV is capable of transiting and providing an Access Performance (m) of up to the following significant wave height.	[1.25m Hs] meter significant wave height
The CTV is capable of providing safe transport from the Service Base Facilities to each WTG at the following minimum speed in up to the following significant wave height.	knots at [1] Hs meter significant wave height
PAX	[12] PAX
A crew finder/man over board system, according to Vietnam standard regulations	263.1
CTV bow deck area (m ²)	> 8m ²
CTV bow deck load (t)	8t at 100% bunker capacity
CTV bow deck load/m ² (t/m ²)	1.0 t/m ²
CTV internal storage (m ³)	6.0 m ³



PECC-2POM



ENVIRONMENT

OBJECTIVES:

To ensure compliance of all environmental statutory requirements

To achieve the above, the following operations are required:

- 1) To make action plans to resolve all environmental issues and implement them
- 2) Monitor all key parameters and report on a regular basis highlighting exceptions
- 3) Maintenance waste management





PECC-2POM

03 0&M Services



PECC-2POM



20

The Second Vietnam Symposium on Advances in Offshore Engineering



PECC-2POM





Equipment for maintenance work such as crane, barge, tools are always available for use



Turbines has ability to remotely restart to start/stop, proactively handle the problem quickly





Organize training courses to improve skills for personnel

Pecc+2*Pom*

Look for different

suppliers





PECC-2POM

MAINTENANCE ACTIVITIES



PECC+2POM

MAINTENANCE ACTIVITIES



MAINTENANCE ACTIVITIES



PECC+2POM







From nearshore to offshore – opportunities and challenges

Riccardo Felici

OWC Vietnam, ABL Group





VSOE From Nearshore to Offshore – Opportunities and Challenges

owcltd.com







VSOE From Nearshore to Offshore – Opportunities and Challenges

owcltd.com





OWC – the Offshore Wind Consultants

Long standing experience of developing and realising projects

- Established in 2011 by a team of experts who had been involved in offshore wind farm construction since 1990s
- Consulted on 68 GW or 82 offshore wind projects since 2011*
- Global specialist offshore wind consultancy with a local presence
 - Experienced technical advisor, engineering consultant and project management consultant, the only global consultancy dedicated to offshore wind
 - >100 offshore wind experts in 8 countries, Supported by c 380 master mariners, naval architects, project managers & engineers in 33 countries
 - Dedicated OWC teams in London, Edinburgh, Hamburg, Warsaw, Taipei, Ho Chi Minh City, Tokyo, Busan, Boston, New York & Australia





4

Providing advisory services across the market







ABL Group - Global Partner, local expert



5

* 884 full-time equivalent employees as of 31 December 2020 ** ABL locate many staff strategically at maritime and offshore hubs to be able to serve clients locally Additional note: the 38 countries number is driven by our offices, in terms of locations where we have surveyors etc we cover 71 countries, a truly global footprint



From Nearshore to Offshore

Opportunities and Challenges

© 2022 OWC



Vietnam wind development

To date majority of wind developments are onshore and nearshore. Primarily nearshore developments have been driven by

- Unique seabed profile and resource opportunities
- Lower costs (both construction and O&M)
- Quicker development and installation

With increasing energy demand, larger scale projects are needed.
Thanks to its world class resources in Vietnam
→ Offshore wind will become a key asset in its renewable energy mix

What are the Opportunities? What are the Challenges? What can we leverage from the country's experience?



Opportunities - Wind Resource

Offshore offers typically:

- Higher Wind Speed

8

- More Consistent wind speed
 - Stable Direction
 - Reduced Turbulence







Opportunities – Scale & Location

Offshore Wind typically offers:

- Much larger turbines
 - Higher wind speed
 - More consistent wind
 - Lower visual impact
 - Lower noise impact
- Larger projects

9

- Availability of sea-area

Ultimately these factors lead to an overall higher energy yield





Challenges – Development

Offshore development presents several differences during the development phase.

- Assessment of resources (FLIDAR vs LIDAR)
- Electrical architecture (offshore transformer or converter station)
- Foundations design (jacket or monopiles)
- Increased number of potential landfall (grid connection points)










Challenges – Installation

Moving offshore increases overall operations complexity due to:

- · Distance to operations port
- Increased wind farm size
- Water depth
- Met-ocean

Result of these factors would lead to a reduced installation window when using methodologies and vessels used for nearshore.

Typically, the use of specialized vessels is needed to achieve higher operability.

- All operations need specialized vessels:
- Foundation Installation
- WTG installation
- Cable installation
- **OSS** Installation

11









Challenges – O&M

Operations and Maintenance (O&M) also require more specialized vessels to be able to access the wind farm year-round.

Typical specialized vessels will be:

- · Offshore Supply Vessels with walk to work systems to access WTG / OSS
- · Cable repair vessels with jointing equipment
- Jack-up vessels for blade replacement

These are not currently present in the Vietnamese market









12



Opportunities

Vietnam has developed several onshore, nearshore windfarms and O&G to date.

Key opportunities can be found in:

- Foundations and OSS construction
- Barges and tugs for transportation
- Offshore supply vessels
- Understanding and development of offshore sites
- Understanding WTG installation
- Specialized personnel
- Ship construction







https://vsoe2021.sciencesconf.org/



Innovation within offshore site investigation

David Donaghy Ocean Infinity, UK



Innovation within offshore site investigation.

David Donaghy – Technical Manager Geotechnics Ocean Infinity 24th October 2022









Innovation within offshore site investigation.

David Donaghy – Technical Manager Geotechnics Ocean Infinity 24th October 2022

Innovation within offshore site investigation.





Innovation within offshore site investigation.

Who are Ocean Infinity?

ŝ

Our team

More than 500 technology and data specialists, including uncrewed vessel operators, ROV pilots, AUV operators, robotics engineers, payload specialists, geotechnical specialists, data processors, surveyors, oceanographers, hydrographers.



We have teams in the UK, Portugal, Norway, Sweden, New Zealand, Australia, Singapore and the USA.

c:O



We use innovative technology, to transform operations at sea, to enable people and the planet to thrive.



Innovation within offshore site investigation.

Our technology.







Next generation operations at sea

We envisage a time, in the not too distant future, where remote and lean-crewed operations at sea will overtake conventional crewed operations.

Technological infrastructure

Integrated digital environment



Remote Control Centres

Our custom developed Remote Control Centres bridge human operators with robotic platforms.

Safe navigation

A new way of working





Marine Robotics

Remotely operated vessels and equipment offer safer and more environmentally responsible operations.



Energy

Offshore data acquisition with minimal environmental impact.







Innovation within offshore site investigation.

Marine & Geotechnical Robotics.



8

 $\dot{\mathbf{O}}$

Armada A-78



Armada A78: a series of 8 vessels, First A78 – Launched in May 2022

- Lean-Crewed vessel operations
- Light and Heavy ROV Work
- · Inspection Maintenance and Repair
- Geotechnical Investigations
 - Shallow and Deep
- Deep Water MBES
- Multi-AUV
- Alternative Fuels highly sustainable

Geotechnical Robotics



Innovation within offshore site investigation.

Some specific challenges we are solving.



ŝ





Manning & Space Requirements







Offshore Sample & Core processing





- Automated core processing and laboratory test scheduling
- X-ray
- Samples stay in tubes

 $\dot{\mathbf{o}}$







Offshore wind power: the potential renewable energy & remarkable aspects

Tran Quoc Dien

Power Engineering Consulting Joint Stock Company 3 (PECC3), Vietnam



VSOE2021 Sustainable Energy and Marine Planning

Offshore Wind Power The Potential Renewable Energy & Remarkable Aspects

Speaker: Tran Quoc Dien – PECC3 Deputy General Director

Contents

O1. Offshore Wind Power - Development Trend
O2. Offshore Wind Power Development – Initial Steps
O3. Remarkable Aspects
O4. Recommendation for Developers

05. PECC3 and Offshore Wind Power Development



EVNPECC3



EVNPECC3



EVNPECC3

EVNPECC3

The Second Vietnam Symposium on Advances in Offshore Engineering

Offshore Wind Power - Development Trend



Offshore Wind Power – Possible Initial Steps

PROJECT IMPLEMENTATION 1. TECHNICAL DESIGN 2. AUTHORITY INSPECTIONS/ **PROJECT PREPARATION** ACCEPTANCES 1. FEASIBILITY STUDY 3. AUTORITY ACEPTANCES 2. INFRASTRUCTURE CONNECTION 4. ECT ... AGREEMENTS 3 AUTHORITIES SELECTION OF INVESTOR ACEPTANCES/APPROVALS 1. PROPOSAL FOR INVESTMENT 4. PPA 2. OTHER PROCEDURES 5. LAN & SEA SURFACE AQUISITION DECISION ON INVESTMENT POLICY 6. OTHER PROCEDURES 1. PRE-FS 2. PROPOSAL FOR SURVEY & PRIMARILY SURVEY OF INVESTIGATIONS **PROJECT AREA** 3. IMPLEMENTATION OF SURVEY 1. DESTOP STUDY, DATA AND INVESTIGATION PURCHASING 2 NATIONAL POWER DEVELOPMENT PLANNING 3. MARITIME SPATIAL PLAN

4. SECTOR PLANS

https://vsoe2021.sciencesconf.org/

Remarkable Aspects EVNPECC: • Power Development Plan 8 Maritime Spatial Plan **LEGAL FRAME WORKS** Sector Plans 42.9 GW • Relevant Resolutions, Laws, Decrees, Circulars ≈13 GW • Technical Instructions (Electrical Price, Power Purchasing, Standards) **REGULATION MECHANISMS** • Procedure Instructions (Approvals, Appraisals, Bidding,..) • Supplement Mechanisms 5 GW 7.28GW SOCIO-ECONOMIC • Electric Price compromising with Economic Development ≈118 GW **SUSTAINABILITY** • Energy Security 40.0 GW COP 26 & Approaching Zero CO2 in 2050 **ENVIRONMENTAL IMPACT** ≈ 26 GW Load Evacuation Schemes Compromising with Sources and Load **Offshore Wind Technical Potential Capacity &** Base Load Demand Scenario, forecasted in 2030 **TECHNICAL** Regionals Source: The last draft NPDP8 Power System Stability

Recommendation for Developers

VNPECC

It is essential to process the procedure and appraisal with prudence.

• Offshore Wind Development should be approached with long-term strategy, requiring thorough preparations from parties;

• Particularly, the government, as well as state management agencies, hold the major responsibility in making decisions, forming the policies that will affect the country's energy security as well as economic and social impacts.

At this moment, all parties should be patient and well-prepare for the next steps.

• Based on the international lesson learnt and knowledge from previous seminars, it is shown that, in the US & Western countries, the energy transition and the initial stage of the Offshore Wind Power Development took a long time.

Especially, the investor should prepare for project technical aspects, start looking for potential suppliers at this stage.

• To compose competitive bidding document and ensure the feasibility of the project by the time the national legal framework and procedures are ready.



EVNPECC3

PECC3 and Wind Power Development

PECC3 have provided consultancy services for 27 Onshore & Offshore Wind Power Projects with total installed capacity of onshore is 1505 MW;

Remarkable Offshore Wind Projects:

- Thang Long Offshore Wind Project: full range of consultancy services
 - Binh Thuan Offshore Wind Project: Survey Permit Proposal and Preliminary Load Evacuation Study;
- Vung Tau Offshore Wind Project: Survey Permit Proposal and Preliminary Load Evacuation Study;
- Thai Binh Offshore Wind Project: Survey Permit Proposal and Preliminary Load Evacuation Study.

Partnerships:

_



Mr. Tran Quoc Dien - Deputy General Director

Please refer our website: www.pecc3.com.vn

Email: Dientg@pecc3.com.vn

Tender Document Preparation
Tender Assistance

Owner Engineering

EPC Management

Site supervision &

Commissioning



THANK YOU!





Offshore wind energy challenges and solutions: a few lessons from nearshore wind power projects in Vietnam

Nguyen Viet-Hung CTE Wind Vietnam







The Annal Annal Annal Annal



SOE The Second Vietnam Symposium on Advances in Offshore Engineering

Offshore Wind Energy Challenges and Solutions : A few lessons from **Nearshore Wind power Projects in** Vietnam



Viet-Hung NGUYEN, **CEO of CTE Wind Vietnam** vh.nguyen@cte-sa.com cell: +84979858540

Web: cte-wind.com

- Hung Nguyen, Ceo of CTV Wind, CTE Wind group who specialize in cBoP design for Wind power Plant with a wide international experience on large scale active in Vietnam.
- Hung NGUYEN received his Ph.D. in Structural Engineering and Materials from the "Ecole des Ponts ParisTech", have nearly 20 vears of experience in the design of cBoP of Wind Power project in Europe, Asia especially in Vietnam
- · He have worked on a wide range of technical design and management including infrastructures, geotechnics, Wind power master plan and specializing for onshore and nearshore Wind **Turbine foundations**
- CTE Wind group has designed more than 2000 wind Farm with the total 20 GW over 74 countries.
- CTV has design more than 60 Wind projects in Operation, 11 near offshore wind Farm















AII CAPEX COST BREAKDOWN

Category	Category cost	Category component	Category component cost	Overall component	Overall component cost		
Development & Consent		Labour	2.4%				
	4%	Materials	0.9%	Labour	05%		
		Other	Labour	35%			
Turbine		Labour	17%				
	33%	Materials	10%				
		Other	6%	Manada	0.49/		
		Labour	9%	Material	34%		
Balance of Plant	37%	Materials	21%				
		Other	6%				
		Labour	6%		2 4 6 4		
Installation & Commissioning	26%	Materials	2%	Other	31%		
		Other	18%				
	100%		100%	1	100%		

Source: BVG Associates



Preliminary Study for foundation types

One preliminary report for choose the foundation type has performed the criteria below:

- Constructability, including the local availability of construction materials and skilled labour;
- Logistics of Installation, Including local Installation contractor experience, availability of Installation equipment, etc.;
- · Cost of manufacturing/construction;
- · Cost of Installation;
- Driveability of driven piles
- Dynamic response (typically only significant in relatively deep water).

The two types best suited to the Project were selected

Pile cap foundation and mono pile foundation

- The cost of solution mono-pile foundation has higher than classical pile cap foundation

NEARSHORE& OFFSHORE FOUNDATION CASE STUDY

- Context
- Site is flooded
- The depth of water is shallow from 3 to 10 m
- The position of foundation is about 500 10km from the shore.
- Seabed is very plane slope < 1- 2%
- The classical foundation has a lot inconvenience
 - The construction cost is very high
 - The quality of construction is not assured
 - · Duration of construction is very long
 -
- It is difficult to user the offshore technical because shallow water
 - Big vessels











(CH) SÉT, MÁU XÁM XANH, XÁM ĐEN, TRANG THÁI CHỦY ĐẾN DỀ (CH) VERY SOFT TO MEDIUM STIFF CLAY, BLUISH GREY

(SC-SM) CÁT PHA BỤ, SÉT, MÀU XĂM VĂNG, KẾT CẦU CHẬT VÀA ĐẾN CHẬT (SC-SM) SILTY, CLAYEY SAND, YELLOWISH GREY, MEDIUM DENSE TO DENSI

(SC-SM) CÁT PHA BU, SÉT



Adapt to Local condition

Materials information

-250

- Local suppliers chains
- Local equipment and local cu
- Local natural and geo eographic
- synergy team bet low and the local



VIII

New Design adapt to the local conditions

IFICATE OF PATENT OF INVEN Number of patent: 17225

NAME OF PATENT: FOUNDATION FOR WIND TURBINE ON THE NEARSHORE ZONE

ORIGINAL ASSIGNEE: CTV INGENIERIE VIETNAM NR: Nguyen Viet Hung, Tran Quang Dat, Nguyer APPLICATION NUMBER: 1-2014-01963 FILED DATE: 16.06.2014 Points reauired for protection: 01 User the local materials

User the local equipment Small barge and equipment

Adapt to real condition

Allow to finished the wind project eventhought the Covid

Profit well the local human ressources



https://vsoe2021.sciencesconf.org/



Geotechnical Conditions

- Type of soil indicates type of foundation
- Dry or flood-risk region
- Complex or homogenous soil
- Number of boreholes needed

Geotech survey program and resulting foundation design are cost relevant



International - local conditions

- Local Regulations and Standards
 - Blended and adapted the international and local regula
 - Standard for design
 - Standard for cconstruction
 - For environment impact

•

							thu	ir hai R	b,ser, R	otser, MI	Pa		-			-	
Trang	Loại bê	Cấp độ bền chịu nén của bệ tông															
thái	tông									<u> </u>							

rờng độ tiêu chuẩn của bệ tông R_m R_m và cường độ tính toán của bệ tông khi tính toán theo các trang thái giới

	tông																				
thái			B 1	B1,5	B2	B2,5	B3,5	B5	B7,5	B10	B12,5	B15	B20	B25	B30	B35	B40	B45	B50	B55	B60
							M50	M75	M100	M150	M150	M200	M250	M350	M400	M450	M500	M600	M700	M700	M800
Nén dọc trục (cường đó lăng	Bê tông nặng, bê tông hạt nhỏ		-	-	-	-	2,7	3,6	5,5	7,5	9,5	11,0	15,0	18,5	22,0	25,5	29,0	32,0	36,0	39,5	43,0
tru) R _{bn} , Ru	Bê tông nhẹ		-	-	-	1,9	2,7	3,5	5,5	7,5	9,5	11,0	15,0	18,5	22,0	25,5	29,0	-	-		-
o,ser	Bê tông tổ ong		0,95	1,4	1,9	2,4	3,3	4,6	6,9	9,0	10,5	11,5	-	-	-	-	-	-	-	-	-
	Bê tông năng		-	-	-	-	0,39	0,55	0,70	0,85	1,00	1,15	1,40	1,60	1,80	1,95	2,10	2,20	2,30	2,40	2,50
	Bê tông hạt nhỏ	nhóm A	-	-	-	-	0,39	0,55	0,70	0,85	1,00	1,15	1,40	1,60	1,80	1,95	2,10	-	-	-	-
Kéo dọc trục R _{etn} , R _{et.ser}		nhóm B	-	-	-	-	0,26	0,40	0,60	0,70	0,85	0,95	1,15	1,35	1,50	-		-	-	-	-
		nhóm C	-	-	-	-	-	-	-	-	-	1,15	1,40	1,60	1,80	1,95	2,10	2,20	2,30	2,40	2,50
	Bê tông nhẹ	cốt liệu đặc			-	0,29	0,39	0,55	0,70	0,85	1,00	1,15	1,40	1,60	1,80	1,95	2,10	-		-	-
		cốt liệu rỗng	-	-	-	0,29	0,39	0,55	0,70	0,85	1,00	1,10	1,20	1,35	1,50	1,65	1,80	-	-	-	-
	Bê tông tổ ong		0,14	0,21	0,26	0,31	0,41	0,55	0,63	0,89	1,00	1,05	-		-	-		-	-	-	-

CHÚ THÍCH 1: Nhóm bê tông hạt nhỏ xem 5.1.1.3.



Local condition: materials

- Concrete
 - WTG foundation needs high concrete grades, which is not very popular in Vietnam, C50/60
- Reinforcement (steel)
 - In general the strength of reinforcement used in Vietnam is less than 400 MPa
 - It better to use high strength grades, for example B500B, but this kind of reinforce is not common in the province
- Grout high strength
 - In Vietnam, we must import the grout. It should be planed to buy it before the construction starts because the importation process takes time



7/2017 11

Pile driving

- Equipment
 - Hammers
 - Diesel hammers
 - Hydraulic hammers

The number of big diameter (more than 1.4m)is very limited

PHC pile diameter more than 1200mm is very limited





Pile broken accident – Case stiff soil





- Control the stabilization of barge for driving.
- The wave height in the driving process is important
- Contact with the head pile is not good
- It should choose the suitable hammer
- Plan to use the cushion



450 600

30 40

FMX (tn) —• Maximum Force

0 150 300

15.0

17.0

19.0

21.0

23.0

25.0

29.0

31.0

33.0

35.0

0 10 20

DMX (mm) —---Maximum Displacement

Ξ

ď 27.0

ġ

Piles broken in soft soil





https://vsoe2021.sciencesconf.org/

TSX (M

6.4

0.0 3.2

15.0

17.0

19.0

21.0

23.0

25.0

27.0

29.0

31.0

33.0

35.0

Ē



Control the temperature for mass concrete

- Use the concrete Low Heat Cement
- Use the cooling system
- Control the slump of concrete
- Use isotherm system



Never allow to moring the badges to the foundation



Conclusion

- It should pay attention about the local condition.
- It should develop the local supply chain about
 - The construction materials for example the high performance of concrete, steel structure, piles
 - Heavy equipment for transportation and installation: heavy transportation vehicles, big crane, driving barge, Vessel, etc..
 - Education and training the human resource for the construction, installation, operation and maintenance
 - Create synergy betwen Vietnameses Companies and Internationals companies

Tra Vinh Ree V 1.3 Wind Farm. 48 MW







Tân Thuận Wind Farm 75 MW



https://vsoe2021.sciencesconf.org/



PHU LAC PROJETS









WIND FARM IN THAILAND



Viet-Hung NGUYEN, CEO of CTE Wind Vietnam vh.nguyen@cte-sa.com cell: +84979858540

Web: cte-wind.com

XIN CẢM ƠN QUÝ VỊ ĐÃ LẮNG NGHE



Installation risks and opportunities for future WTG foundations

Daniele Bertalot Geowynd, Italy





Installation Risks and Opportunities for Future WTG Foundations

Dr. Daniele Bertalot, Principal Geotechnical Engineer, Geowynd









Installation Risks and Opportunities for Future WTG Foundations

Dr. Daniele Bertalot, Principal Geotechnical Engineer, Geowynd

Content of the Presentation

- Overview of WTG Installation Operations
- Jack-up related (geotechnical) installation hazards
 - > Example 1: Assessment of jack-up vessel response upon soil liquefaction
 - > Example 2: Assessment of pile driving induced soil softening
- Pile related (geotechnical) installation hazards
 - > Example: Performance of SRD methodologies applied to Monopile foundations
- Conclusions





Overview of WTG Installation Operations

Most of WTG foundations in the South-East Asia region are/will be:

- o Monopiles
- Pin-pile Jackets
- Jackets on suction buckets

In most cases installation operations are carried out from jack-up vessels (few DP installation vessels also available)

Pile foundations (both Monopiles and Pin-piles) are typically installed by impact driving (limited cases of vibro-driving)



Credit: Ground Engineering, Hornsea II OWF

Jack-up Related (Geotechnical) Installation Hazards



<u>Jack-up related hazards</u>, concern with the stability of the installation vessel during operations. Jack-up stability is routinely addressed by means of:

- Leg Penetration Analyses
- Site Specific Assessment (Yield Interaction, Footing Eq. Stiffness,...)

ISO 19905-1 also stresses the need to check jack-up stability against cyclic loads, in cases where foundations soils are potentially subject to 'cyclic mobility':

9.4.6 Cyclic mobility

Cyclic loads can cause a progressive build-up of pore pressures within the foundation soils and consequent soil strength degradation (liquefaction). The effects can be either local to the soils under the spudcan or over a larger area. Local foundation cyclic loading can be caused by the jack-up response to earthquakes, severe storms, rotating machinery, etc. Earthquakes can cause large-scale cyclic loading and result in failure of the soil mass over a larger area. Depending on the magnitude of pore pressures developed, cyclic loading can result in large vertical displacements of the spudcans, which can be differential in some cases.

The assessment shall consider the effects of cyclic loading on the stability and displacements of foundations.


Cyclic Mobility: Seismic Loading



Open issue: Lack of guidance in the definition of a <u>reference risk level</u> for WTG installation operations (i.e. Return period of the seismic action):

- \rightarrow ISO 19901-2 treating jack-up as a manned, fixed, offshore structure (very conservative)
- \rightarrow ISO 19905-1 recommends ELE=1000 yrs for O&G jack-up drill rigs, still applicable to wind industry?
- \rightarrow DNV RP 0585 substitutes 'service life' with 'construction window' in ISO 19901-2 (ELE>95 yrs)



Cyclic Mobility: Seismic Loading

Modelling approach:

- 1. Definition of the 2D (or 3D) FE model geometry including at least 30 m soil column
- 2. Definition of seismic input in the form of in-profile acceleration time histories at model base level
- 3. Material model calibration based on cyclic laboratory element tests (Fully coupled up formulation required)

- → Undrained dynamic analysis (conservative)
- → Dynamic analysis + consolidation





Ŷ



Cyclic Mobility: Seismic Loading



Cyclic Mobility: Pile Driving Induced Vibrations

During driving of pile foundations part of the energy of the hammer stress wave is radiated away from the pile in the form of shear and volume waves:

- 100.00 L = 35 m L = 50 m -----60.00 Hammer Pile 222222 Cylindrical shear waves: generated at the pile-soil interface, Vs, decay with a rate proportional to 1/R Spherical volume waves: generated at the pile-soil interaface,
- Vp of about 1500 m/s, decay with a rate proportional to 1/VR
- Rayleigh waves, surface waves with both transverse and longitudinal motion components, ~ Vs







Cyclic Mobility: Pile Driving Induced Vibrations



Cyclic Mobility: Pile Driving Induced Vibrations





Pile Related (Geotechnical) Installation Hazards



<u>Pile related hazards</u>, concern with the safe installation of the pile to target penetration.

Main pile driving hazards are:

- Pile refusal
- Tip extrusion buckling (e.g. boulder impact)
- Driving induced steel fatigue
- 'Pile running'

Risk for specific installation is typically assessed by means of 1-D wave equation analysis of the hammer-pile-soil system:

Reliable Soil Resistance to Driving (SRD) profile is key

SWZ MARITIME Home News Events Subscribe SWZ Archive

Heerema's Aegir suffers dropfall incident while piling in Taiwan



RECHARGE



Giant turbine foundation lost at sea as new setback hits major Taiwan offshore wind project

Soil Resistance to Driving

- 20 installation case histories
- 3 Offshore Wind Farms
- 10 Monopiles
- 10 Pin-piles
- Soil conditions considered:
 - ✓ Very dense to medium dense sands
 - ✓ Slightly to heavily over consolidated clays

SRD Method	Y'	qc	q _t	fs	U2	φ′	δ	Su	St	YSR	Ip	UCS
Alm and Hamre [2]	~		~	~			~					
Stevens et al. [33]	~						~	~		~ ^	v ^	~
Fugro-modified Toolan and Fox	~	~		~			~	~				
Maynard et al. [25]	~		~	~			~			~	~	
Jones et al. [21]	~	~		~		√ ∧	~		~	~		

	No. of		Ra	ange of Pile (
Method	Piles	Bound	D [m]	L [m BSF]	<i>t</i> [mm]	L/D [-]	D/t [-]	Range of Ground Conditions
Alm and	164	Min.	0.76	35.0	-	14.4	-	Typical North Sea conditions: dense to very dense sands, low to
Hamre [2]	104	Max.	2.74	115.0	-	150.9	-	extremely high strength clays, lightly to heavily overconsolidated.
Stevens et al.	27	Min.	0.91	17.4	38.1	53.4	20.6	Arabian Gulf carbonate soils: medium dense to very dense
[33]	27	Max.	1.07	50.6	44.5	181.5	28.0	strength clays, gypsum and sandstone rock.
Toolan and	22	Min.	1 27	59.7	50.0	43.5	27.0	Forties Bravo Platform, North Sea:
Fox (1977)*	52	Max.	1.37	67.1	50.8	48.9	27.0	to extremely high strength clay.
Maynard et	202	Min.	4.80	23.6	50	3.8	57.5	Predominantly very high to extremely high strength clays and
al. [25]	202	Max.	7.50	46.5	100	7.3	130.0	dense to very dense sands across three sites.
Jones et al.	277	Min.	0.66	20.8	35.0	3.3	17.4	Ranges from normally consolidated clays and silts offshore China to
[21]	2//	Max.	6.50	126.0	82.7	131.8	130.0	overconsolidated clays in the North





Soil Resistance to Driving



Soil Resistance to Driving





Conclusions

Few key points to be considered for future installation of WTG foundations in Vietnam:

- Site characterization for future OWF shall focus on identifying soils
 potentially affected by cyclic mobilty/liquefaction, and carefully determine
 their response to cyclic loading (i.e. adequate cyclic laboratory program)
- For OWF projects located along Vietnam's Eastern an Northern coasts, the impact of <u>earthquake loading</u> on jack-up stability shall be carefully assessed, including cyclic mobility effects
- The <u>risk level to be considered for jack-up vessels</u> in the wind industry is an open issue, ultimately up to the operator/certifier to define acceptable risk
- <u>Uncertainty related to SRD</u> predictions from industry standard empirical methods shall be considered when assessing pile installation of large diameter monopiles









Thank You for the Kind Attention

Please feel free to reach out for additional details dbe@geowynd.com



Potential development of floating offshore wind turbine in Vietnam offshore

Thanh-Dam Pham

Duy Tan University, Vietnam



Potential Development of Floating Offshore Wind Turbine in Vietnam Offshore

Thanh-Dam Pham^{1, 2}, Du Van Toan³, Thi-Khang Nguyen³, Hyunjeong Ahn⁴, Hyunkyoung Shin^{4*}, Quoc Sy Pham⁵

1 Institute of Theoretical and Applied Research, Duy Tan University, Hanoi, 100000, Viet Nam 2 Faculty of Natural Sciences, Duy Tan University, Da Nang, 550000, Viet Nam 3 Vietnam Institute of Seas and Islands, Hanoi 123075, Vietnam 4 University of Ulsan, Ulsan 44610, South Korea 5 IPC Engineering & Construction JSC (IPC E&C)





Potential Development of Floating Offshore Wind Turbine in Vietnam Offshore

Thanh-Dam Pham^{1, 2}, Du Van Toan³, Thi-Khang Nguyen³, Hyunjeong Ahn⁴, Hyunkyoung Shin^{4*}, Quoc Sy Pham⁵

1 Institute of Theoretical and Applied Research, Duy Tan University, Hanoi, 100000, Viet Nam 2 Faculty of Natural Sciences, Duy Tan University, Da Nang, 550000, Viet Nam 3 Vietnam Institute of Seas and Islands, Hanoi 123075, Vietnam 4 University of Ulsan, Ulsan 44610, South Korea 5 IPC Engineering & Construction JSC (IPC E&C)

Outline

- Introduction
- Reference Location and Environmental Conditions
- 10 MW Floating Offshore Wind turbine Models
- Numerical Simulation Modelling
- Results
- Conclusion





Introduction



Floating offshore wind platform concepts



Source: COWI

3 Turbi 26 MiN

1

- Shared supply chains
 - Turbines
 - Array and export cables
 - Regulations
 - Ports and Infrastructure
- Operations and Maintenance
- Floating cost reductions lag fixedbottom offshore wind cost by 5 -7 years
- Floating cost are likely to converge with fixed-bottom wind





Figure credit: NREL



Introduction



Introduction

LCOE of floating offshore wind









Reference Location and Environmental Conditions

Technical potential capacity of offshore wind: **599 GW** Fixed bottom offshore wind is **261 GW**

Floating offshore wind is 338 GW





Source:Worldbank, "Technical Potentia for Offshore Wind-in Vietnam Map"

Reference Location and Environmental Conditions



Ninh Thuan, Binh Thuan Offshore: Water depth from 50-250 m, area~8500 km2 Wind speed 10.6 m/s at 100m height

Reference Location and Environmental Conditions

Typical environmental conditions at Ninh Thuan offshore area based on data from Vietnam Institute of Seas and Islands

Sea	Wind	Wa	ave	Cumont	DLC
State	wind	Hs	Тр	Current	DLC
	m/s	m	s	m/s	
SS1	4	0.12	2.9	0.4	DLC1.1
SS2	6	0.47	2.9	0.4	DLC1.1
SS3	8	0.63	2.9	0.4	DLC1.1
SS4	10	1.05	3.68	0.4	DLC1.1
SS5	12	1.33	3.88	0.4	DLC1.1
SS6	14	1.81	4.51	0.4	DLC1.1
SS7	16	2.36	5.08	0.4	DLC1.1
SS8	18	3.01	5.74	0.4	DLC1.1
SS9	20	3.79	6.38	0.4	DLC1.1
SS10	22	4.71	7.18	0.4	DLC1.1
SS11	24	5.6	7.72	0.4	DLC1.1
SS12	4 - 24	8.8	10	0.4	DLC1.6a
SS13	42.1	8.8	10	1.47	DLC6.1a

Reference DLC

U			
Item	DLC 1.1	DLC 1.6a	DLC 6.1a
Wind model	NTM	NTM	EWM
Sea state	SS1-SS11	SS12	SS13
Wind and wave directionality	09 COD	0° COD	$MIS\pm 30^{o}$
which and wave unectionality	0°, COD	0°, COD	Yaw $\pm 8^{\circ}$, 0°
Current model	NCM	NCM	ECM
W/indextains states	Orientier	Onerting	Parked,
wind turbine status	Operation	Operation	blades:900

Note: COD is co-directional, ECM is extreme current model, ESS is extreme sea state, EWM is extreme wind speed model, MIS is misalignment of wind and wave, NTM is normal turbulence model

10 MW FOWT Models

10 MW Offshore Wind Turbine



Description	Value
Rated power of wind turbine	10-MW
Rotor orientation	Upwind, 3 blades
Control	Variable Speed, Collective Pitch
Rotor diameter [m]	178
Hub height [m]	120
Rated wind speed [m/s]	11.2
Rated rotor speed [rpm]	9.6 (gearless)
Hub mass [kg]	169,440
Hub inertia about shaft [kg⋅m²]	829,590
Nacelle mass (target) [kg]	335,000





10 MW FOWT Models

Platforms: semi-submersible and spar





Semi-submersible

Principle of platform design:

Rated pitch angle < 5 degree,

Heave and pitch natural periods are out of ocean wave period range Semi-submersible: upscaled from 5 MW semi-sub DeepCwind model Spar: upscaled from 5 MW OC3 Hywind model, but keep the same draft 120m



		_	
Description	Unit	Semi-sub	Spar
Volume Displacement	m3	25888.63	16056
Center of Buoyancy below SWL	m	-16.18	-62.133
Platform Mass	ton	25,297	14512
Center of Mass (CM) of Platform below SWL	m	-16.56	-92.31
Platform Roll Inertia about CM	kgm ²	1.922E+10	9.87E+09
Platform Pitch Inertia about CM	kgm ²	1.922E+10	9.87E+09
Platform Yaw Inertia about CM	kgm ²	3.452E+10	3.42E+08

10 MW FOWT Models



Description	Unit	Ser	ni-Submers	Spar		
Water depth	m	50	150	250	150	250
Mooring diameter	mm	162	142	130	130	111
Mooring line mass density (air)	kg/m	576.8	443.2	371.5	371.5	270.8
Axiall stiffness (EA)	MN	2362.0	1814.8	1521.0	1521.0	1108.9
Unstretched mooring line length	m	500.0	800.0	1050.0	800.0	1100.0
Depth to fairleads below SWL	m	-17.2	-17.2	-17.2	-48.0	-48.0
Radius to fairlead	m	51.0	51.0	51.0	7.0	7.0
Radius to anchor	m	543.0	810.6	979.5	785.0	1057.2
Minimum Breaking Load (MBL)	MN	18.2	14.7	12.7	12.7	9.6

Principle of mooring design:

- · The same pretension, the same mooring line angle at fair lead
- · Touchdown length is equal or bigger to ensure there will be no lift up force at anchors
- Mooring lines are studless chain class R3





Numerical Simulation Modeling



Results



https://vsoe2021.sciencesconf.org/





Conclusion

- Vietnam offshore area is suitable for deploying floating wind farms.
- Semi-sub and spar models work well and stable in operation conditions and extreme environmental conditions at different water depths.
- Considering to installation, semi-submersible model is preferable for the deployment of floating wind farms.





Thank you!



https://vsoe2021.sciencesconf.org/



AI-based framework to predict wave-induced settlement of offshore wind turbines

Amir Moghaddam

School of Engineering, RMIT University, Australia



AI-based framework to predict waveinduced settlement of Offshore Wind Turbines

Amir Moghaddam, Amin Barari School of Engineering, RMIT University, Melbourne, VIC 3000, Australia







AI-based framework to predict waveinduced settlement of Offshore Wind Turbines

Amir Moghaddam, Amin Barari School of Engineering, RMIT University, Melbourne, VIC 3000, Australia

Introduction





Solving algorithm of WSFI



FE model setup





z=-0.5m

z=-4.5m

Calibration of CycLiqCPSP parameters



3 different relative densities



Simulation of cyclic direct simple shear (CDSS) test in OpenSees

60

Validation of numerical model



Simulation of centrifuge wave tank test in OpenSees







Results from numerical analyses

Development of EPWP around the caisson







Results from numerical analyses

- Development of EPWP around the OWT foundation
- Influence of initial stress and plots of stress path







Results from numerical analyses

- Development of EPWP around the caisson
- Influence of initial stress and plots of stress path
- Bearing capacity and degraded bearing capacity





Results from numerical analyses

- Development of EPWP around the caisson
- Influence of initial stress and plots of stress path
- Bearing capacity and degraded bearing capacity
- Influence of wave characteristics on bearing capacity ratio







Results from numerical analyses

- Development of EPWP around the caisson
- Influence of initial stress and plots of stress path
- Bearing capacity and degraded bearing capacity
- Influence of wave characteristics on bearing capacity ratio
- Influence of various model configuration on caisson settlement





Results from numerical analyses

-0.1 0.0 -0.2 -0.2 -0.4 Settlement (m) -0.6 Settlement (m) H=1m H=2m -0.3 -0.8 H=3m H=4m H=5m -1.0 Over 250 FE dynamic -0.4 -1.2 -1.4 analyses with the aid of -0.5 -1.6 H=1 H=10 high performance -1.8 -0.6 40 12 50 60 70 0 16 Dr (%) $t_{s,l}(m)$ computing (HPC) facilities **(b) (a)** 0.0 -0.1 -0.5 -0.2 Settlement (m) -1.0 Settlement (m) ← H=1m H=2m → H=3m H=1m -0.3 H=1m H=2m H=3m H=4m H=5m H=6m H=7m -1.5 H=5m H=5m H=6m H=7m H=8m H=9m H=10n -0.4 -2.0 + H=8m -0.5 -2.5 H=10 -3.0 -0.6 0.0 0.2 0.4 0.6 0.8 1.0 0 100 200 300 400 500 E_r O (kPa) (c) (d)



Non-linear multi-variable regression

$$Sw = a/(1 + be^{-cD_{r}})$$

$$Sw = a + bt_{s,l} + ct_{s,l}^{2} + dt_{s,l}^{3} + et_{s,l}^{4}$$

$$Sw = q (1 + bE_{r}/a)^{(-1/b)}$$

$$Sw = a + bQ$$

$$Sw = a + bD_{r} + ct_{s,l} + dE_{r} + eQ + fH$$

$$R^{2} = 0.716 \quad !!!!$$



Group Method of Data Handling (GMDH)

$q_1 = -0.1522 - 0.0483D_r - 0.0717E_r - 1.9004 \times 10^{-4}D_r^2 - 3.8269E_r^2 + 0.1512D_rE_r$	·····	[] [] []	1
$q_2 = 0.4003 - 0.0915t_{s,l} + 0.2540E_r - 0.0150t_{s,l}^2 - 4.1192E_r^2 + 0.5468t_{s,l}E_r$	D_r		
$q_3 = 0.7832 + 0.3916E_r - 0.0070Q - 3.8721E_r^2 + 1.0916 \times 10^{-6}Q^2 + 0.0115E_rQ$			
$q_4 = -1.4123 + 4.5882E_r - 0.1637H - 3.8962E_r^2 - 0.0010H^2 + 0.2531E_rH$	t _{s,l}	x_2 q_2	
$S_{\rm tr} = 0.0055 + 0.7044a_{\rm s} + 0.2051a_{\rm s} - 0.2023a_{\rm s}^2 - 0.5432a_{\rm s}^2 + 0.6998a_{\rm s}a_{\rm s}$	Er	x_3 q_3 s_w	
\mathbf{p}^2	Q		
R = 0.931	Н	x_5	
	ił		;
¥\$QE		Inputs Layer I Ou put	



Conclusion

- 1) Wave-seabed-foundation interaction is modelled to investigate wave-induced liquefaction and wave-induced settlement of OWT caisson foundation.
- 2) Non-linear multi-variable regression cannot predict the waveinduced caisson settlement rationally, due to complexity of problem and high scatteredness of data
- 3) A neural network-based method is put into practice known as Group Method of Data Handling (GMDH) using the repetition of a series of multilayered perceptron type network structures to estimate wave-induced caisson settlement with high accuracy









Role of geotechnics in risk management of offshore windfarm projects

Indrasenan Thusyanthan

Gavin & Doherty Geosolutions, Ireland



Role of Geotechnics in Risk Management of Offshore Windfarm Projects

Dr I Thusyanthan | Managing Director FICE CEng CMarEng MEng PhD (cantab) Gavin & Doherty Geosolutions



YSOE

The Second Vietnam Symposium on Advances in Offshore Engineering



Role of Geotechnics in Risk Management of Offshore Windfarm Projects

Dr I Thusyanthan | Managing Director FICE CEng CMarEng MEng PhD (cantab) Gavin & Doherty Geosolutions



Offshore Wind Roadmap for Vietnam (WBG)





Role of Geotechnics in an Offshore Windfarm Project



Role of Geotechnics in Risk Management of Offshore Windfarm Projects







- Stages 1, 2 and 3 are associated with desk top studies, geotechnical and geophysical survey implementation, and geotechnical testing.
- Desk top studies provide an early indication as to the likely seabed conditions to be expected at the project site.
- During site surveys, seabed soils would be subjected to in in-situ testing and seabed soil samples would be further characterised by laboratory testing.
- The number of intrusive geotechnical tests required to characterise the offshore windfarm site would depend on several factors such as geology, accuracy and resolution of the available geophysical survey data, geohazards etc., and hence it cannot be generalised.
- In general, soil conditions cannot be assumed to be uniform across a site unless proven with geotechnical and geophysical data. Therefore, knowing the seabed soil stratigraphy at two locations does not necessarily enable one to interpret the soil stratigraphy between those two locations.
- Since intrusive geotechnical testing has a high impact on project schedule and cost, it is often the case that the number of test locations is optimised to save cost and time. A geotechnical consultant is best place to advise on this optimisation as if not undertaken properly this can lead to increased project risk.









Geotechical data from Survey & Lab testing together with Geophysical survey data are key to ensure that we understand the wind farm site and enables us to design foundations cost effectively







- BS5930 and ASTM D2487 are the most commonly used standards in the industry for soil classification. But BS 5930 and ASTM D2487 follow different methodologies in the way they classify soils.
- BS 5930 (2020) states that, where a soil (omitting any boulders or cobbles) "sticks together when wet and remoulds" it is described as a fine soil ("CLAY" or "SILT" dependent on its plasticity) and when soil does not stick together and remould, it is described as a coarse soil ("SAND" or "GRAVEL" dependent on its particle size grading).
- As per ASTM D2487, if more than 50% of the soil is retained on No. 200 sieve (0.075mm), the soil is classed as a coarse-grained soil, and if 50% or more passes the No. 200 sieve, the soil is classed as a fine-grained soil. Note that the particle size boundary between fine soils and coarse soils is different in these standards, in BS 5930 it is 0.063mm whereas in ASTM D2487 it is 0.075mm.





Soil Classification - BS5930 vs ASTM D2487





Thusyanthan, N. I. (2018), "Insight into On-bottom stability, DNV RP F109 and DNVGL RP F114", Offshore Technology Conference 2018
 Thusyanthan, N.I. (2012), "Seabed Soil Classification, Soil behaviour and Pipeline design", Offshore Technology Conference 2012





- Windfarms can cover hundreds of square kilometers of seabed, hence ensuring that WTGs and cables are not exposed to geohazards both during their installation and operational lifetime is vital. Below are the most common geohazards that need to be considered during design and installation;
 - Seafloor Slope
 - Landslides and earthquakes
 - Faults
 - Presence of boulders
 - Shallow gas
 - □ Channel System (palaeochannels)
 - Environmentally Sensitive Areas
 - Manmade Hazards (e.g.. Unexploded Ordnance (UXO))
- These geohazards need to be identified early in the project timeline and all identified risks need to be managed using appropriate risk mitigation mechanisms. This may include a modified design, micrositing WTGs locations or re-routing the cable routes, change of installation procedure etc. It is acknowledged that some risks can never be fully eliminated, and residual risks may need to be managed through design or during construction.







Foundation Design – Geotechnical Knowledge

Geotechnical insight in critical to be able to design cost effective foundations for the offshore windfarms

As we now reply heavily on Numerical Analyses for designs, we need to ensure that we fully understand the numerical models and the software we use. All numerical models have limitations and we need to ensure we are fully aware of them.











There are three common pile installation methods

SURVEYS

Driven pile

DESK TOP STUDIES &

- Drive-Drill-Drive (3D) or
- Drilled & Grouted.
- Seabed soil conditions determine which method is the most cost effective and have the lowest risk in a particular project as summarized below



SURVEY SCOPING Conceptual studies & survey specifications	Geophysical & Geotechnical Surveys undertaken at Site	TESTING Soil properties from in-situ & lab testing	CHARATERISATION Soil classification & design parameters	DESIGN Design & Risk Identification	Ŋ	WTGs & Cables installation technical support	

All Wind Turbine Generators (WTGs) are connected via inter-array cables and linked to an offshore substation via terminal cables. The offshore substation is then connected to an onshore substation via export cables.

GEOTECHNICA

- All cables need to be installed at the correct burial depth to ensure they are protected from natural and man-made damage risks. Cable Burial Risk Assessment (CBRA) guidance offers a standardized and qualitative method to ensure that cable burial depths are determined consistently using a risk-based approach.
- Trenchability or ploughability assessment for cables needs to be undertaken during design stage to identify risks associated with installation.

	Seab	ed type						
very soft clays, loose sand	s sands a	& clays	Rock					
Fluidisation	Dredging	Jetting/Ploughing	Drilling/Cutting					
Seabed Intervention Methods								





INSTALLTION



Conclusion

- If ground risks are allowed to pass on in project stages, its impact and consequences on project cost and programme can be very high at later stages. Thus, ground risks should be identified as soon as possible and mitigated early on through site investigation, ground modelling and design.
- Ground risks in the project needs to be identified and mitigated by the early engagement of a specialist geotechnical consultant. In-depth knowledge of site surveys, geotechnical testing, seabed classification, foundation designs and seabed interventions are paramount for successfully derisking offshore wind projects.
 - "Unmanaged geotechnical risks can have escalated impact on project cost and programme. Geotechnical consultant's role in ground risk identification and mitigation is paramount for successful offshore wind project"



Dr I Thusyanthan | Managing Director FICE CEng CMarEng MEng PhD (cantab) Gavin & Doherty Geosolutions ithusyanthan@gdgeo.com





Investigation of the long-term cyclic behaviour of monopile foundation by impact and vibratory installation

Le Viet Hung Technische Universität Berlin, Germany



Investigation of the long-term cyclic behaviour of monopile foundation by impact and vibratory installation

Viet Hung Le, Frank Rackwitz Technische Universität Berlin - Germany





Investigation of the long-term cyclic behaviour of monopile foundation by impact and vibratory installation

Viet Hung Le, Frank Rackwitz

Technische Universität Berlin - Germany



Table of contents

- Introduction
- Calculation concept
- Numerical model and validation
- Results





Aims

- Cost reduction of offshore pile foundations
- · Serious alternative to impact driving
- Prediction of long-term loading behaviour





The second Vietnam Symposium on Advances in Offshore Engineering Page 3



Challenges and simplification in FE numerical model

- Complicate soil behavior during pile driving
- After installation: Change of stress condition and density
- Large deformation by penetration process
- Calculation time and error by large number of cycles

The second Vietnam Symposium on Advances in Offshore Engineering Page 4




Calculation concept



The second Vietnam Symposium on Advances in Offshore Engineering Page 5



Soil condition after vibratory and impact diving



The second Vietnam Symposium on Advances in Offshore Engineering Page 6





Explicit calculation for high cyclic loading



The second Vietnam Symposium on Advances in Offshore Engineering Page 7



Model set up



The second Vietnam Symposium on Advances in Offshore Engineering Page 8





Validation of the numerical model



The second Vietnam Symposium on Advances in Offshore Engineering Page 9



The second Vietnam Symposium on Advances in Offshore Engineering Page 10



Changes in vertical displacement





The second Vietnam Symposium on Advances in Offshore Engineering Page 11



Conclusion

- Successful validation of the numerical model
- "Wished in place" => larger deformation
- Vibratory driving pile has less deformation as impact pile

The second Vietnam Symposium on Advances in Offshore Engineering Page 12





Thank you

https://vsoe2021.sciencesconf.org/





Optimisation of capacity predictions for driven Piles performance in carbonate silts for offshore structures in the Arabian Gulf

Muhammad Bilal Mumta Fugro Survey Middle East, UAE







Optimisation of Capacity Predictions for Driven Piles Performance in Carbonate Silts for Offshore Structures in the Arabian Gulf

Muhammad Bilal Mumtaz (Email: m.mumtaz@fugro.com), 24th October 2022

Introduction

- Industry standards provide guidance on dealing with sands (drained) and clays (undrained) soils.
- Lacking guidance on dealing with intermediate soils such as silts.
- Typical practice is to either model silt as drained (frictional) or undrained (cohesive) – dependent on the engineer.
- Carbonate Silts are commonly found in the Arabian Gulf.

https://vsoe2021.sciencesconf.org/

• 4 platform locations in the Arabian Gulf further examined.



UGRO





UGRO

The Second Vietnam Symposium on Advances in Offshore Engineering



Axial Capacity Prediction

- Method A: API RP2 GEO main text guidelines.
 - Cohesive soils: α method based on undrained shear strength ratio
 - Frictional silica soils (CaCO3<20%): β method
 - Frictional carbonate soils (>80% CaCO3): Kolk (2000) approach with f_{lim} =15 kPa.
 - Carbonate silt (CaCO3 >80%): either frictional or cohesive (depending on geotechnical contractor's experience)
 - Intermediate carbonate soils (20% < CaCO3 < 80%): Logarithmic interpolation
- Method B: Adjusted Karlsrud et al. 2005 and Clausen et al. 2005:
 - Cohesive soils : α method based on undrained shear strength ratio and plasticity
 - Frictional silica soils (CaCO3<20%): CPT based approach
 - Frictional carbonate soils (>80% CaCO3): based on effective stress and strength degradation with depth
 - Carbonate silt (>80% CaCO3): same as frictional carbonate soils
 - Intermediate carbonate soils (20% < CaCO3 <80%): Logarithmic interpolation
- Method A and B provide capacity for 100 days after installation.

4



Pile Monitoring

- Axial pile capacities can be estimated using signal matching analyses (most commonly CAPWAP) during driving
- Somewhat user dependent
- Restrike tests can result in estimates of set-up
- CAPWAP analysis results were available at the four selected locations
 - Restrike tests ranging between 1 24 hours







Results

6

- Comparison of measured pile capacities at end of driving and restrike (at 1 to 24 hours after end of driving) with pile capacities predicted by two different methods.
- Predicted capacities consider the silt as frictional.
- Set up measurements likely underestimated compared to 100 days capacity.
- Capacities are underestimated in the deeper silt layer, this is attributed to high shaft resistance.



Discussion



Conclusions

To help address unconventional soil types the following is recommended:

- Carry out pile monitoring with minimum restrike after 24h
- Compare results from different methodologies with pile monitoring measurements
- Consider pile monitoring data in similar soil conditions to adjust engineering predictions
- Develop site specific methods when adequate pile monitoring records are available
- Additional scope:

8

- Investigate the potential for cyclic degradation
- Explore alternative pile testing techniques to improve measurements accuracy
- Combine measurements from structure, metocean and pile foundations to improve accuracy







https://vsoe2021.sciencesconf.org/



Experimental investigation of wave scattering around a large vertical circular cylinder

Tri Mai

Hanoi University of Civil Engineering, Hanoi, Vietnam





Experimental Investigation of Wave Scattering Around a Large Vertical Circular Cylinder

Tri Mai Hanoi University of Civil Engineering, Hanoi, Vietnam









Experimental Investigation of Wave Scattering Around a Large Vertical Circular Cylinder

Tri Mai

Hanoi University of Civil Engineering, Hanoi, Vietnam



Content

- Objectives
- Physical model test
- Phase-based harmonic separation method
- Scattered wave field
- Conclusions





Objectives

• To investigate experimentally wave scattering around a truncated cylinder model to get a better understanding of how wave steepness effects on the local scattered wave field, may affect local and global loads, which may cause damage to or failure of offshore structures.



Physical model test

• The test was carried out in the Ocean Basin at Plymouth University.





Focused wave groups: kA = 0.13 & 0.18(The NewWave methodology with an underlying JONSWAP spectrum ($\gamma = 3.3$)) Water depth: 2.93 m Sampling rate of wave gauges: 128 Hz Cylinder: Diameter of 0.3 m and height of 0.3 m; Draft = 0.15 m; D/L = 0.09



Phase-based harmonic separation method

An incident wave group that has amplitude A and relative phase θ can be expressed as the classic Stokes perturbation expansion (Fenton, 1985):

$$\eta(A,\theta) = B_{11}A\cos\theta + A^2(B_{20} + B_{22}\cos 2\theta) + A^3(B_{31}\cos\theta + B_{33}\cos 3\theta) + A^4(B_{40} + B_{42}\cos 2\theta + B_{44}\cos 4\theta) + O(A^5)$$
(1)

where B_{ij} are the coefficients in Fourier series for $\eta(A,\theta)$; *i* is the amplitude content order; and *j* is the frequency content order.

$$\eta(A,\theta) = \eta_{11} + (\eta_{20} + \eta_{22}) + (\eta_{31} + \eta_{33}) + (\eta_{40} + \eta_{42} + \eta_{44}) + O(A^5)$$
(2)

where, η_{ij} are the *j*th-order harmonic components, $\eta_{ij} = A^i B_{ij} \cos(j\theta)$.

If *i* and *j* are identical, then η_{ij} are the *j*th-order harmonic sum, e.g. the first-order sum η_{11} . If *i* and *j* are different, then η_{ij} are the *j*th-order harmonic difference, e.g. the term η_{31} is at the first-harmonic in frequency but 3rd order (cubic) in input wave amplitude.

-> the subscript *i* is referred to as the (amplitude) order and *j* as the harmonic.

Phase-based harmonic separation method

The separation method (Fitzgerald et al., 2014):

$$\frac{\eta^0 + H(\eta^{90}) - \eta^{180} - H(\eta^{270})}{4} = \eta_{11} + \eta_{31}$$
(3)

$$\frac{\eta^0 - \eta^{90} + \eta^{180} - \eta^{270}}{4} = \eta_{22} + \eta_{42} \tag{4}$$

$$\frac{\eta^0 - H(\eta^{90}) - \eta^{180} + H(\eta^{270})}{4} = \eta_{33} \tag{5}$$

$$\frac{\eta^0 + \eta^{90} + \eta^{180} + \eta^{270}}{4} = \eta_{20} + \eta_{40} + \eta_{44} \tag{6}$$



The linear combinations of time histories and the <u>Hilbert transforms</u> of the 90° and 270° free-surface elevation time histories are applied!



Phase-based harmonic separation method



Phase-based harmonic separation method





Scattered wave field

The scattered or diffracted wave field:

$$\eta_{ij}^{dif} = \eta_{ij}^{Model} - \eta_{ij}$$

Where: η_{ij}^{dif} is the $(i,j)^{th}$ component of the scattered/diffracted wave η_{ij}^{Model} is the equivalent component with the model present η_{ij} is the undisturbed incident wave component measured at the same gauge location in the absence of the model



Scattered wave field





Scattered wave field



Scattered wave field





Conclusion

- This experimental study have investigated the effects of wave steepness on the scattered wave field around a truncated cylinder;
- A general phase-based harmonic separation method has been applied;
- There is a significant effect of wave steepness on the second and third harmonic components where the amplitude of those higher harmonic components increases with increasing the wave steepness;
- The fourth harmonic component at the bow of the cylinder is smaller than that at the locations around the cylinder;
- The second, third and fourth harmonics should be considered in practical design of offshore structure for assessment of springing- and ringing-type responses and the air gap.







CÁM ƠN! THANK YOU!



https://vsoe2021.sciencesconf.org/



Case studies on mitigating pile foundation refusals

Sumanth Haribhat Chandrashekhar

Sarathy Geotech & Engineering Services Pvt Ltd, India







Sarathy Geotech & Engineering Services Pvt Ltd sumanth@sarathygeotech.com





Case Studies on Mitigating Pile Foundation Refusals



Sarathy Geotech & Engineering Services Pvt Ltd sumanth@sarathygeotech.com





A. GEOTECHNICAL INVESTIGATION

B. PRE INSTALLATION DRIVEABILITY STUDY

C. PILE MONITORING DURING DRIVING







PRE INSTALLATION DRIVEABILITY STUDY

- INPUT
 - Hammer properties
 - Hammer and pile cushion properties
 - · Pile properties
 - Soil Properties
- OUTPUT
 - Resistance Vs Blow Count
 - Stress vs pile length
 - Transferred Hammer Energy



PRE INSTALLATION DRIVEABILITY STUDY





AXIAL, BENDING AND DRIVING STRESSES IN PILE - P5 ADD-ON ~ 11.5m MHU 1700 Hammer, 85% Hammer Efficiency 1.524m (60-in) OD Open Ended Steel Pipe Piles











Driving of Pile with Hydraulic Hammer



Pile Monitoring Using PDA



INTRODUCTION



- Assessment of as installed pile capacity is required for acceptance of offshore piles refused prematurely at a penetration shallower than design penetration
- Piles refused prematurely generally satisfy the requirement of compression capacity but not the tension capacity and the minimum penetration requirements for lateral stability
- In such cases, the refused pile is advanced to deeper penetrations by drill and drive technique up to the design penetration
- The following case studies highlight the importance of assessing the as installed capacity during the drill and drive process for control over drill-out length and the use of pile driving records for back analysis of pile capacity.







CASE STUDY-1 Persian gulf



CASE STUDY-1



- Steel pipe piles of outer diameter 1.524m and varying wall thickness of 31.75-88.90mm formed the foundation piles for a four-legged offshore jacket platform installed in the Persian Gulf.
- The IHC500 hydraulic hammer was proposed to drive the piles through the jacket legs to the final penetrations ranging 88-95m below the seabed.

0 -12m	Very Soft CLAY
12.0 – 76.5	Stiff to Very Stiff CLAY
76.5 – 110.8	Hard CLAY

- An intermittent 2.8m thick, dense cemented carbonate SILT is present at 41.5m, followed by 0.3m thick weak crystalline GYPSUM at 44m.
- Pile drivability analysis, performed for the estimated soil resistance to driving using the soil parameters, indicated no premature refusal.







- The first driven pile, Pile A2, encountered refusal at 44.50m. No decision could be taken on pile acceptance at refused depth, as the pile was not instrumented and monitored and so with all the remaining piles.
- > Pile toe buckling suspected due to hard driving and driving was stopped
- About a year later, in the second pile driving campaign, pile A2 was instrumented and found the pile toe was intact, and the compression and tension capacity was not sufficient for acceptance
- CAPWAP (Case Pile Wave Analysis for Piles), a signal matching software, estimated compression and tension capacities were 26MN and 5.6MN, respectively, while the design capacities were 68 MN and 58 MN at design penetration of 95m.







 CPT data of Gypsum layer indicated a refusal tip resistance of more than 50MPa.





Fig 1: Borelog Excerpt



A pilot hole of 1.2m diameter was drilled up to 1.0m below the pile toe. The core cuttings stuck in the drill brit confirmed that the material encountered was grey Gypsum.



Fig 2: Drill Bit



Fig 3: Drilling in Progress











VSOE

Pile driving resumed after the drill-out. It was hard driving again with a restart blowcount of 300blows/0.25m because of the Gypsum below the pile wall area.

The CAPWAP computed compression capacity after the drill-out of Gypsum was **20MN** vis-à-vis **26MN** before drill-out.

The reduction in capacity was due to reduction in skin friction as evident in the Force-Velocity vs Time graphs as shown in below.











SGES

Although the end bearing was still high, since the pile toe was intact and computed capacity was within the mobilized capacity during refusal, pile driving was continued without resorting to another round of plug drill-out below pile toe which otherwise would have caused more reduction in pile capacity.

- The blowcounts gradually reduced to 50blows/0.25m after about 1m drive, and the pile could be driven to design penetration successfully.
- It was concluded that the Gypsum layer thickness was 1.3 to 2.6m vis-à-vis 0.3m reported in the borelog. The strength of Gypsum was underestimated with an end bearing of 1.13MN vis-à-vis mobilized end bearing of 18MN from CAPWAP.
- Predicted vs Computed capacity, Blowcount vs penetration are shown in figures 6-7



Fig 6: Predicted Capacity vs. Computed Capacity



Fig 7: Blowcounts vs. Penetration







Case STUDY-2 Offshore West Coast India





- Four Legged jacket platform, located in Mumbai High Field. Foundation piles of 2.134m OD with varying wall thickness 38-74mm.
- Hydraulic hammers IHC S-600, IHC S-900, and IHC S-1200 were proposed for pile driving. Three piles were driven to their design penetration of 116.6m below the seabed, while the fourth pile, pile B1, had met with premature refusal at 113.5m depth.
- It was required to assess the as-installed pile capacity for acceptance. However, piles were not instrumented, hence, Pile driving records viz., blowcounts, and energy readings from the hammer console were used for pile capacity assessment
- GRLWEAP (Wave Equation Analysis for Piles) was used in simulating the hammer-pile-soil system and pile capacity was back analyzed using a technique called Blow Count-Depth Matching (BCDM) [Rausche F et al, 2009, Gangisetti R et al, 2020],

VSOE





- The back analyzed asinstalled capacity was 45.4MN with skin friction and end bearing components of 22.6MN and 22.8MN.
- The modified unit skin and end bearing values vs Penetration is shown Figure 8.



Fig 9: Blow count-Depth Matching (GRL WEAP)



- Blowcount-Depth matching is shown in Figure 9
- The back analyzed capacity of 45.4MN for the end-of-driving (EOD) condition was less than the design capacity of 66.16MN.
- Pile monitoring equipment was mobilized for instrumentation, and a re-strike test was conducted using IHC-S900 hammer on pile B1, 3.8 days after the pile had experienced the refusal driving conditions.







- The CAPWAP computed pile capacity was 56.6MN with skin friction and end bearing components of 47.18MN and 9.3MN.
- As the hammer energy during re-strike test was sufficient only to mobilize skin friction, full end bearing could not be mobilized. Therefore, the back analyzed end bearing for the end-of-driving condition was added to the skin friction mobilized during the re-strike.
- The total as-installed capacity of 69.78MN (47.18MN+22.6MN) during re-strike test was more than the design capacity of 66.16MN; hence the pile was accepted at 113.5 penetration depth of refusal.
- The Force-Velocity vs Time graph from PDA and CAPWAP results and The back analyzed EOD capacity, CAPWAP Re-strike capacity and Design capacities [API RP-2A, 2000] are shown Figures 10-11





CONCLUSIONS



- Most of the premature pile refusals can be attributed to unexpected lateral variation in the strata and their thickness.
- Cemented hard silt with crystalline Gypsum stains or weak Gypsum rock can cause refusal conditions if its strength is underestimated during pile drivability studies.
- If the capacity requirements are not met, then plug drill-out can be done to advance the pile to design penetration.
- Plug drill-out done in stages with capacity assessment using CAPWAP will avoid over drill-out, which otherwise would cause a significant capacity reduction.
- If instrumented data is not available, the capacity can be back analyzed using pile driving records and GRLWEAP, although less accurate than CAPWAP using instrumented data.



REFERENCES



- American Petroleum Institute, "A Recommended Practice for Planning, Designing, and Constructing Fixed Offshore Platforms," (RP 2A-WSD), Twenty-First Edition, December 2000, Errata And Supplement 1, December 2002.
- Colliat, J.L., Vergobbi, P., Puech, A. (1993), "Friction degradation and set-up effects in hard clays offshore Congo and Angola." Offshore Technology Conference, OTC 7172.
- Rauche F., Nagy M., Webster S., Liang L. (2009), "Capwap and Refined Wave Equation Analysis For Driveability Predictions And Capacity Assessment Of Offshore Pile Installations." Proceedings of the ASME 28th International Conference on Ocean, Offshore and Arctic Engineering, OMAE2009.
- Ramesh Gangisetti, Dr. CR Parthasarathy (2020), "Evaluation of offshore pile capacity and pile integrity using dynamic pile monitoring services." Advances in Offshore Geotechnics pp 161-176, Indian Symposium on Offshore Geotechnics, ISOG 2019.







sign of Deep Excavation with Nails,

29 -Driven piles -Offshore piles We are the authorized technical representatives of PILE DYNAMICS, Inc. USA (PDI) in India

Exclusive Partners for YJACK, Malaysia in India region - World-wide Patented Technology.

& G

Pile Monitoring Activi

Estir

tomography







Thank You

SARATHY GEOTECH & ENGINEERING SERVICES PVT LTD (SGES) (An ISO 9001:2015 & ISO 45001:2018 Certified)

> Have you Visited us Today? www.sarathygeotech.com



Tra Vinh No.3 windfarm: CBOP package - lessons learned

Pham Duc Huyen FECON, Vietnam



Tra Vinh No. 1.3 Wind Farm Project CBOP Package - Lessons Learned

Pham Duc Huyen Head of Dept. of Civil & Industrial Construction No. 3 FECON CORPORATION

VSOE

The Second Vietnam Symposium on Advances in Offshore Engineering



Tra Vinh No. 1.3 Wind Farm Project CBOP Package - Lessons Learned

Pham Duc Huyen

Head of Dept. of Civil & Industrial Construction No. 3 FECON CORPORATION



The Second Vietnam Symposium on Advances in Offshore Engineering

CONTENTS

- 1. BRIEF ON TRA VINH NO.3 WINDFARM
- 2. MOBILIZATION OF RESOUCES
 - 2.1 EQUIPMENT MOBILIZATION
 - 2.2 THE WEATHER CONDITION AFFECTED
- 3. QUALITY MANAGEMENT
 - 3.1 PILLING WORKS
 - 3.2 MASS CONCRETE CONTROL
- 4. STAKEHOLDER MANAGEMENT



1. BRIEF ON TRA VINH No.3 WINDFARM

- Location: Duyen Hai townTra Vinh Province
- Investor: REE Corporation
- Capacity: 48 MW with 12
 Turbines of Vestas
- EPC Contractor: Vestas
- CBOP Contractor: FECON





1.BRIEF ON TRA VINH No.3 WINDFARM




1. BRIEF ON TRA VINH No.3 WINDFARM



1. BRIEF ON TRA VINH No.3 WINDFARM





Formwork and rebar work



1. BRIEF ON TRA VINH No.3 WINDFARM



Floating batching plant-Concreting work

1. BRIEF ON TRA VINH No.3 WINDFARM





Link Bridge U-beam installation



9

The Second Vietnam Symposium on Advances in Offshore Engineering

1. BRIEF ON TRA VINH No.3 WINDFARM



2. MOBILIZATION OF RESOUCES



Test piling work

 Using inappropriate construction equipment will affect the progress and quality of the project







2.1 EQUIPMENT MOBILIZATION



The suitable equipment for the marine piling work is the most important to make sure the progress, safety and quality.



2.2 EQUIPMENT MOBILIZATION

Hydraulic hammer selection for construction

- Selection of construction equipment based on technical calculations and construction experience.
- ✓ Closely monitor the construction process.
- Check the frequency by the necessary experiment.
- The project encountered many serious construction quality errors, due to the use of inappropriate construction equipment





2.3 THE WEATHER CONDITION AFFECTED

Every year, the windy season begins in the southern at the end of October and will last until the end of March. During this season, the waves up to 5m high can occur at times, putting any equipment at sea in danger.

The project's progress should be adjusted accordingly to avoid falling into the windy season, or construction must be suspended to ensure the project's safety.





3. QUALITY MANAGEMENT

2.1 PILING WORK:

Experiences in driving PHC piles at sea:

- Choosing inappropriate equipment (hydraulic hammer) will cause:
- damage to piles (shown by low BTA)
- additional cost and slow progress.





Pile damaged at the connection joint due to hammer blows dropped in a short period time.





3. QUALITY MANAGEMENT

CHALLENGES IN PILING

Some challenges in piling at the site

- ✓ Severe weather conditions (storms, high waves) hamper the foundation work significantly
- Large barges cannot be deployed in shallow waters, so small barges is appropriate. Small barges, on the other hand, is vulnerable to high waves and strong wind
- Supplies can easily be interrupted by severe weather conditions. Pile material could not be transported to the site due to this.

EXPERIENCES IN PILING

Some recommendations to minimize pile damages

- ✓ The proper hammer type and capacity are critical factors to keep pile integrity. Using highload hydraulic hammers can cause severe damage to PHC piles.
- ✓ Diesel hammers are found better than the hydraulic hammers and are generally recommended.



3. QUALITY MANAGEMENT

2.2 MASS CONCRETING OF FOUNDATION







3. QUALITY MANAGEMENT

TEMPERAURE CONTROL IN MASS CONCRETING

CONCRETE CRACK FORMATION vs CORE TEMPERATURE Δ



Cracking due to insufficient insulations



→ Selecting correct insulation & correct removal time is crucial importance for mass concreting

17

3. QUALITY MANAGEMENT

TEMPERAURE CONTROL IN MASS CONCRETING Key methods to control temperature

- Control of mix design (cement content, W/C ratio)
- Use of different lifts
- Proper insulation methods and proper removal time
- Installation of cooling system (if any)





Principle of temperature reduction by cooling pipe



An example of FEM analysis with cooling pipes installed



19

The Second Vietnam Symposium on Advances in Offshore Engineering

3. QUALITY MANAGEMENT



MASS CONCRETING AT WT No.9



Curing concrete surface (covered by wetted jute bags and watered daily)

4. STAKEHOLDER MANAGEMENT



21

The Second Vietnam Symposium on Advances in Offshore Engineering

4. STAKEHOLDER MANAGEMENT

- Fishermen may be drawn into the construction site, potentially endangering both equipment and people. To ensure the project's security and safety, it is necessary to coordinate and support local border guards.
- The process of obtaining a maritime safety permit can be time-consuming, so

getting access to Maritime authorities as soon as possible should also be a priority.







Highlights from R&D on innovative monopile installation and decommissioning

Ahmed Elkadi

Research Program Manager: Energy Transition, Deltares, Netherlands







Highlights from R&D on innovative monopile installation & decommissioning

Ahmed Elkadi, PhD, CEng Deltares, Delft, Netherlands







Deltares



Highlights from R&D on innovative monopile installation & decommissioning

Ahmed Elkadi, PhD, CEng Deltares, Delft, Netherlands





<complex-block>

https://vsoe2021.sciencesconf.org/



Monopiles





Deltares



Deltares

VSOE2021

SIMOX (Sustainable Installation of XXL Monopiles)

- Start date 1st June 2021
- 3 years project; 2021 2024
- 6.5 M€; of which ~4 M€ government subsidy



Deltares

Project Objectives (WHAT)

SIMOX will investigate, collect, and develop the necessary technical and environmental knowledge with the aim of having <u>one or more qualified and</u> validated next-generation installation technologies available within 5 years.

This is by testing various installation techniques, of which the degree of technical development differs from each other, namely

- the technologies that utilize vibratory driving with a purely vertical excitation;
- the Gentle Driving of Piles concept, which is an emerging technology that combines a vertical vibratory excitation with a high-frequency torsional shaking;
- the VibroJet technique, combining controlled inner jetting with axial vibration

Additionally, the project will assess the implications for decommissioning at the end of service life.

Deltares



VSOE2021

5

6

s mox

Installation Technologies



Axial Vibro Tool https://www.cape-holland.com/



torsio **GDP** Shaker vibrations https://growoffshorewind.nl/project /gentle-driving-of-piles





VibroJet https://www.gbmworks.com



Impact Hammer (As reference case) https://www.ihciqip.com/en/product s/piling-equipment/hydrohammer





VSOE2021

7

Deltares

SIMOX test campaign





SIMOX test campaign - scaled lab testing

Filling tank with excavator



Compaction with vibrating needles



Deltares

SIMOX test campaign – scaled lab testing

- Per tank: 2 impact, 6 vibro or 3 impact, 5 vibro
- Variables:
 - Impact: fall height, weight
 - Vibro: frequency, lowering speed of crane











SIMOX test campaign – scaled lab testing



Deltares

Decommissioning offshore wind farms

- Almost 30% of the total installed wind turbine capacity in Europe is older than 15 years
- By the end of service life, wind farm operators need to decide between the extension of the asset lifetime, repowering or <u>decommissioning</u> the site
- Nevertheless, regardless the final service life of a wind turbine, decommissioning will always happen

Deltares



Hydraulic Pile Extraction – sustainable decommissioning



Deltares

Deltares

JIP HyPE-ST (Hydraulic Pile Extraction-Scale Tests)



Identify, understand and quantify **soil-pile-interaction mechanisms** during application of **hydraulic overpressure** to extract a (mono)pile under different boundary conditions in a controlled environment (small-scale)

VSOE202

13

VSOE2021



Larger scale: Water-Soil-Flume (Water-Grond-Goot)





Preparing clay bed



VSOE2021

16

VSOE2021



Preparing sand bed



Test piles



Pile installation



Deltares

Hydraulic extraction - measurements







20

VSOE2021

https://vsoe2021.sciencesconf.org/



Overview

Deltares



Hydraulic extraction - experiment





VSOE2021

22

VSOE2021





Ahmed Elkadi ahmed.elkadi@deltares.nl

- www.deltares.nl
- @deltares
- info@deltares.nl
- @deltares
- in linkedin.com/company/deltares
- f facebook.com/deltaresNL

